

Treatability Study Work Plan Permeable Reactive Barrier Pilot, Revision 2 Nevada Environmental Response Trust Site, Henderson, Nevada

Prepared for: Nevada Environmental Response Trust

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Treatability Study Work Plan, Permeable Reactive Barrier Pilot, Revision 2

Nevada Environmental Response Trust (Former Tronox LLC Site) Henderson, Nevada

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Nevada Environmental Response Trust (Former Tronox LLC Site) Henderson, Nevada

Responsible Certified Environmental Manager (CEM) for this project

I hereby certify that I am responsible for the services described in this document and for the preparation of this document. The services described in this document have been provided in a manner consistent with the current standards of the profession and, to the best of my knowledge, comply with all applicable federal, state and local statutes, regulations and ordinances.

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Acronyms and Abbreviations

-	
AMPAC	American Pacific Corporation
AWF	Athens Road Well Field
bgs	below ground surface
BMI	Black Mountain Industrial
COC	contaminant of concern
COPCs	chemicals of potential concern
DO	dissolved oxygen
ENVIRON	ENVIRON International Corporation
FBR	fluidized bed reactor
ESTCP	U.S. Department of Defense, Environmental Security Technology Certification Program
FRTR	Federal Remediation Technologies Roundtable
ft	feet
ft/ft	feet per foot
GEO	geochemistry sampler
GC-FID	gas chromatograph – flame ionization
gpm	gallon per minute
GWETS	Groundwater Extraction and Treatment System
IC-MS/MS	Ion chromatography-mass spectroscopy-mass spectroscopy
ISM	in-situ microcosm
ITRC	Interstate Technology & Regulatory Council
IWF	Interceptor Well Field
MI	Microbial Insights, Inc.
MICRO	Bio-Trap [®] sampler
mg/L	milligrams per liter
mL	milliliter
NDWR	Nevada Division of Water Resources
NAC	Nevada Administrative Code
NDEP	Nevada Division of Environmental Protection
NERT	Nevada Environmental Response Trust
NOI	Notice of Intent
NPDES	National Pollutant Discharge Elimination System

Northgate	Northgate Environmental Management, Inc.
ORP	oxidation-reduction potential
PID	photoionization detector
PPE	Personal Protective Equipment
PRB	Permeable Reactive Barrier
PVC	polyvinyl chloride
Qal	quaternary alluvium
QA/QC	control assurance/quality control
qPCR	quantitative polymerase chain reaction
RAOs	Remedial Action Objectives
SERDP	Strategic Environmental Research & Development Program
Shaw	Shaw Environmental Inc.
Site	NERT Site
SOPs	Standard Operating Procedures
SWF	Seep Area Well Field
Tronox	Tronox LLC
µg/L	micrograms per liter
UIC	Underground Injection Control
UMCf	Upper Muddy Creek Formation
µSm/cm	microSiemens per centimeter
USA	Underground Service Alert
USDW	Underground Source of Drinking Water
USEPA	United States Environmental Protection Agency
VFAs	volatile fatty acids
WBZs	water-bearing zones

1 Introduction

ENVIRON International Corporation (ENVIRON) on behalf of the Nevada Environmental Response Trust (NERT) has prepared this Treatability Study Work Plan for a Permeable Reactive Barrier Pilot for the Nevada Division of Environmental Protection (NDEP). This Treatability Study Work Plan provides a scope of work, including in-situ microcosm (ISM) testing, bench-scale column testing, and pre-design activities to enable design of a permeable reactive barrier (PRB) pilot system to be developed to treat perchlorate-impacted groundwater at the NERT Site in Clark County, Nevada (the "Site"). ENVIRON is currently investigating potentially feasible technologies to be used in conjunction with the existing Groundwater Extraction and Treatment System (GWETS) at the Site. Various in-situ and ex-situ technologies are under consideration to mitigate the migration of perchlorate in groundwater. Of the technologies currently under consideration, in-situ treatment through the use of PRB technology appears to represent a particularly promising method to reduce current and future costs of the GWETS while providing an effective means to mitigate perchlorate migration from the Site. If effective, the PRB-emplaced treatment could help to reduce the need for downgradient extraction of groundwater and treatment in the GWETS as is currently performed at both the Athens Road Well Field (AWF) and the Seep Well Field (SWF), and thereby, significantly reduce the cost for remediation of the perchlorate groundwater plume at the Site.

This Treatability Study Work Plan provides details on the initial proposed studies, including ISM studies and bench-scale column studies, to provide information to aid the design of a PRB pilot system at the Site. Although a discussion is included as to what a PRB pilot system may entail, the information presented is preliminary and is based on the current knowledge of the Site. The design and monitoring of such a PRB pilot system will be refined as more information is collected in the ISM and column studies.

This Work Plan has been prepared and is being submitted as part of the Remedial Investigation (RI) and Feasibility Study (FS) for the Site, pursuant to the Interim Consent Agreement entered into by the Trust effective February 14, 2011. A RI/FS Work Plan to address soil and groundwater contamination at the Site was submitted to NDEP on December 27, 2012. The RI/FS Work Plan was reviewed by NDEP and various stakeholders during 2013 and a revised work plan, addressing and incorporating comments from NDEP and stakeholders was submitted to NDEP on January 10, 2014. NDEP provided comments on the RI/FS Work Plan on April 25, 2014, with revisions due for submittal to NDEP by May 25, 2014. The RI/FS Work Plan and this revised Treatability Study Work Plan are anticipated to be reviewed by NDEP during May and June 2014. Implementation of this work plan is dependent on NDEP approval of the work plan and associated budgetary approval.

1.1 Background and Regulatory Status

1.1.1 Groundwater Contamination

The Site has been undergoing active remediation to manage hexavalent chromium groundwater contamination (since 1986) and perchlorate contamination of groundwater (since 1998), under consent orders issued by NDEP to the Kerr McGee Chemical Corporation and since February 14, 2011, pursuant to an Interim Consent Agreement between NERT and NDEP. Both constituents are treated by means of a groundwater extraction system and on-site treatment

facilities, collectively referred to as the GWETS. Groundwater is collected at three well fields: the on-site Interceptor Well Field (IWF), the off-site AWF, and the off-site SWF. Groundwater collected from the IWF is first treated to reduce hexavalent chromium to trivalent chromium through a ferrous sulfate treatment system. After the ferrous sulfate treatment process, perchlorate is treated using perchlorate-reducing bacteria in a series of fluidized bed reactors (FBRs). Groundwater extracted from the AWF and SWF is discharged directly to the FBR process for perchlorate removal. Following treatment, groundwater is discharged to the Las Vegas Wash under a National Pollutant Discharge Elimination System (NPDES) permit (NV0023060).

The on-site IWF also includes a bentonite-slurry barrier wall, which was constructed in 2001 as a physical barrier across the higher concentration portion of the on-site perchlorate groundwater plume. The barrier is approximately 1,600 feet (ft) in length and 60 ft deep, constructed to tie into approximately 30 ft of the underlying Upper Muddy Creek Formation (UMCf).

Although the current GWETS has effectively removed substantial amounts of perchlorate (and hexavalent chromium) from groundwater, elevated concentrations persist in groundwater at the Site.

1.2 Work Plan Organization

This Work Plan document relates to the proposed in-situ microcosm testing, column testing, and pre-design activities necessary for design and installation of a PRB pilot system and is organized as follows:

- Section 2 presents the purpose and objectives of the proposed PRB pilot;
- Section 3 presents the Site conditions in the candidate location of the proposed PRB pilot;
- Section 4 presents an overview of PRB technology and the rationale for the proposed PRB pilot;
- Section 5 presents the proposed approach for design of the PRB pilot, including up-front soil boring, well installation, in-situ and bench-scale studies, establishment of design parameters, and reporting;
- Section 6 presents a preliminary monitoring scheme to be undertaken for the PRB pilot treatability study;
- Section 7 presents the proposed schedule for the studies; and
- Section 8 details the references used in compiling this Work Plan.

Tables and Figures are presented at the back of the report text, followed by the Appendices.

2 Purpose and Objectives

2.1 Purpose

As described in Section 1.1, the GWETS is currently in operation at the Site. The GWETS extracts and treats groundwater containing perchlorate and hexavalent chromium to control the migration of these chemicals of potential concern (COPCs) in groundwater and to limit the discharge of COPCs to the Las Vegas Wash. The purpose of this Work Plan is to evaluate the technical feasibility and overall effectiveness of an in-situ PRB in treating perchlorate to levels that will achieve the remedial action objectives (RAOs) for perchlorate in groundwater at the Site. To properly evaluate this technology ENVIRON proposes to conduct ISM testing and column studies, followed by installation and operation of a PRB pilot system at the Site. The specific objectives for these studies including a summary of work done to date (by others) are provided below.

2.2 Objectives

The objective of the ISM studies, bench-scale column tests, and pilot test is to evaluate the effectiveness of using PRB technology as a component of the overall remediation of the Site. The ISM testing will provide valuable information to assess the performance of various amendments under actual in-situ conditions. The bench-scale column testing will provide supplemental information regarding the degradation of perchlorate under laboratory-controlled conditions. The ISM and bench-scale studies collectively will develop necessary information required for the design and implementation of a full-scale PRB at the Site, which could be used for sustained in-situ treatment of perchlorate in groundwater to meet RAOs. This will be achieved by the specific objectives presented below.

2.2.1 Bio-Trap® ISM Testing Objectives

Laboratory bench-scale studies have traditionally been employed to screen potential in-situ bioremediation strategies. However, duplication of in-situ conditions in the laboratory is difficult and the results may not correlate directly to the field. For this reason, ENVIRON proposes to supplement the bench-scale study described below with a method known as Bio-Trap[®] ISMs, to asses and screen bioremediation strategies directly in the field. ISMs are a cost-effective method to supplement laboratory studies to provide the chemical, geochemical, and microbiological lines of evidence required for screening remediation options and validating selected remediation technologies. ISM studies consist of an assembly of physically isolated units, each corresponding to a specific option, such as monitored natural attenuation, biostimulation and/or bioaugmentation. Each ISM unit contains multiple passive diffusion samplers to examine COPC concentrations, redox conditions, and microbial populations. The assembly of ISM units can be deployed in existing monitoring wells and is recovered for subsequent analysis to simultaneously screen multiple treatment options. Evaluation of remediation alternatives is based on comparisons of multiple lines of chemical, geochemical, and microbiological evidence between the ISM units.

ISMs allow the evaluation of various bioremediation approaches under actual in-situ conditions in a single-field study by simultaneously deploying multiple ISM units in a single-test well. The below illustration and photograph show the components and a picture of an ISM unit, respectively. To evaluate the effect of various treatment approaches, the ISM would be

equipped with three types of passive samplers: a contaminant of concern (COC) sampler, a geochemistry (GEO) sampler, and a Bio-Trap[®] (MICRO) sampler. The COC sampler results are used to compare concentrations of parent compounds (perchlorate) and daughter product formation (chlorate, chlorite, and chloride) between ISM units undergoing different treatments. Quantification of geochemical parameters including competing electron acceptors (nitrate, sulfate, etc.) and volatile fatty acids (VFAs) are used to compare redox states and provide the geochemical footprint of subsurface microbial activity in each ISM unit.

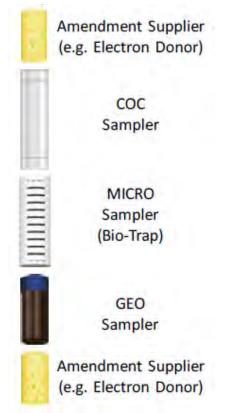


Illustration of an ISM sampler including COC sampler, GEO sampler and MICRO sampler.



Photograph of a MICRO sampler assembly.

The MICRO samplers contain a proprietary sampling matrix, Bio-Sep[®] beads, which are readily colonized by subsurface bacteria. Following ISM deployment, MICRO samplers are recovered from each ISM unit for quantitative polymerase chain reaction (qPCR) enumeration of key microbial populations (e.g., perchlorate reducers). Therefore, comparison of qPCR results for the MICRO samplers in each ISM provides a quantitative assessment on the efficacy of each treatment tested to stimulate growth of organisms responsible for contaminant biodegradation. The qPCR approach has been developed and used in field applications for more than a decade (Higuchi, Dollinger, Walsh, and Griffith, 1992).

The purpose of the ISM evaluation is to obtain information concerning the particular amendment that would be most successful at enhancing the indigenous microbial population to biodegrade perchlorate under native hydrogeologic groundwater conditions. The results of this testing program will identify which microbial populations predominate in the presence of various carbon donors, and how the donor amendment affects the geochemical conditions of the groundwater.

The objectives of the ISM study are as follows:

- Conduct an initial screen to evaluate a variety of electron donors to determine which amendment(s) show the most promising results for biodegradation of perchlorate in Site groundwater;
- Based on the results of the initial screen, use the most promising amendment to establish in situ biodegradation rates for perchlorate in groundwater; and

• Develop the necessary parameters from the observed reaction kinetics to enable the selection of the configuration (e.g., trench PRB, injected PRB) and sizing for design of the PRB pilot system.

2.2.2 Bench-Scale Column Study Objectives

Bench-scale testing using site-specific groundwater and soil cuttings in flow-through columns is proposed to supplement the information obtained in the ISM study for the evaluation of candidate amendments tailored to the Site conditions. The purpose of the column tests is to obtain perchlorate biodegradation rates with the selected carbon donor source and evaluate the potential for fouling or clogging of the aquifer materials with the amendment. The flow-through columns will be monitored throughout the study for influent and effluent perchlorate concentrations, electron acceptor (e.g., oxygen, nitrate, iron, and sulfate), and metals concentrations along with oxidation-reduction potential (ORP) and pH. Since a known amount of perchlorate will be injected into the columns and the amount of perchlorate and perchlorate daughter products will be monitored in the effluent at various locations throughout the column, a mass balance can be performed on the change in perchlorate concentration over time and distance within the column. These results along with the data obtained from the ISM evaluation will be used to guide the design of the PRB pilot system.

The objectives of operation of the bench-scale study are as follows:

- Develop the parameters from the observed degradation rates for design of the PRB pilot;
- Evaluate the performance of the columns under various flow rates; and
- Evaluate conditions that may result in fouling/clogging.

2.2.3 PRB Pilot Objectives

The objectives of the PRB pilot are as follows:

- Evaluate the effectiveness, implementability, and operational limitations (e.g., biofouling) of the design configuration and amendment selected from the ISM Study under actual field conditions at the Site;
- Determine the appropriate dose rates for the selected amendment;
- Evaluate the potential geochemical impact from operation of the PRB pilot system on the solubility and mobilization of metals within the aquifer; and
- Evaluate the hydraulic performance of the PRB pilot system and develop the geotechnical parameters necessary for the design and installation of a full-scale system at the Site.

2.3 Work Performed By Others

Between 2000 and 2010, a series of studies were undertaken and plans were prepared relevant to the application of PRB technology, including the following:

Date	Type of Study or Plan	Type of Study or Plan Performed by	
12/19/2000	Hydrogeologic	Errol L. Montgomery and Associates Inc.	
1/18/2001	Seep Groundwater Characterization	Kerr-McGee Chemical, LLC	
2/14/2010	Work Plan for PRB Pilot Testing	Shaw Environmental, Inc. (Shaw)	
10/25/2010	Emulsion Retention Testing and Bench- Scale Jar Testing	Northgate Environmental Management, Inc. (Northgate)	

A detailed summary of the work performed to date by others related to the proposed PRB pilot is provided in Table 1. In February 2011, the Trust assumed ownership of the Site, following which the Trust and NDEP discussed the implementation of a RI/FS at the Site. As a result, the Northgate and Shaw plans were not implemented and it was agreed that any treatability studies would be evaluated and proposed as part of the RI/FS. ENVIRON has reviewed the prior work plans along with associated NDEP comments and has incorporated relevant details into this Work Plan. The proposed pilot testing herein continues and builds on the preliminary evaluation of PRB technology and proposed pilot testing previously presented by others.

3 Site Conditions

3.1 Geology

From review of available borehole logs (Northgate, 2011) and as is described in the following, the geology of the area of the proposed PRB is comprised of the following three units: general fill, quaternary alluvium (Qal) and a Tertiary UMCf.

- **Fill Material** is not generally present in the area of the proposed PRB, the exceptions being in borehole MW-K5 (northeastern corner of the proposed PRB area) and PC-103 (adjacent to the southwestern corner of the proposed PRB). In these areas, fill is described as a silty sand (3.5 ft thick) overlying a clayey, sandy gravel to 8 ft below ground surface (bgs) (MW-K5); and as "construction material" (taken to refer to demolition rubble) extending to 6 ft bgs (PC-103).
- Quaternary Alluvium is present in each of the seven locations drilled to date in the area of the proposed PRB and generally comprises a reddish-brown heterogeneous mixture of wellgraded sand and gravel with lesser amounts of silt and clay. The gravel comprises the aforementioned Tertiary volcanic rocks with rare cobbles encountered (PC-98R at 29–30 ft bgs). Caliches (hardened deposits of calcium carbonate) are also known to be present in the area and were recorded as a band of gravel from 16–20 ft bgs in PC-98R. The alluvial deposits extend to between 29 and 40.5 ft bgs with thicknesses ranging between 23 and 40.5 ft. These alluvial deposits are further described as being loose and coarse (Errol L. Montgomery & Associates, 2000).
- A major feature of the alluvial deposits is the stream-deposited sands and gravels that were laid down within paleochannels that were eroded into the surface of the UMCf during infrequent flood runoff periods. These deposits vary in thickness and are narrow and linear. These generally uniform sand and gravel deposits exhibit higher permeability than the adjacent, well-graded deposits. In general, these paleochannels trend northeastward (ENSR, 2006).
- Tertiary UMCf underlies the alluvial deposits and is comprised generally of gray/green sandy and silty clay to clayey sand with gypsum crystals. This formation was encountered in all but one of the boreholes drilled in the proposed PRB area (borehole I-2 drilled by Northgate as a PRB test bore in 2011, which terminated in the alluvial deposits). Referencing the available borehole logs for the proposed PRB area (Northgate, 2011), the UMCf was encountered between 29 and 40.5 ft bgs. The full thickness of the UMCf was not determined as all the boreholes drilled into it terminated within the first few feet.

Soil boring logs and well construction diagrams for wells in the vicinity of the candidate PRB location are included in Appendix A. A table of well construction details is provided in Table 2. Cross sections showing the detailed geology in the area of the proposed PRB pilot are presented on Figures 3 to 5.

3.2 Hydrology

Depth to groundwater in the candidate PRB pilot area ranges from about 21 to 24 ft bgs. The groundwater gradient averages 0.02 feet per foot (ft/ft) south of the AWF, flattening to 0.007 ft/ft just south of the SWF (ENVIRON, 2011b, 2012). The groundwater flow direction at the Site is

generally north to north-northwesterly. This generally uniform flow pattern may be modified locally by subsurface alluvial channels cut into the underlying UMCf, the on-site bentonite-slurry groundwater barrier wall, off-site artificial groundwater highs or "mounds" created by the infiltration of City of Henderson wastewater effluent discharged to ponds in the Henderson Bird Viewing Preserve, and by depressions created by the groundwater extraction wells at the three groundwater recovery well fields (Northgate, 2010).

The rate of groundwater movement in the area of the candidate PRB location has been estimated previously to be in the range of 30 to 45 ft/day (Errol L. Montgomery & Associates, 2000). Recent groundwater modeling performed by ENVIRON has resulted in estimates of groundwater velocity in the immediate vicinity of the candidate location for the Quaternary Alluvium of approximately 15 ft/day. Given the importance of groundwater velocity to the design and evaluation of the PRB pilot, as discussed in Section 5.2.3 (Single Borehole Dilution Testing) additional testing will be conducted at the candidate PRB location to provide a better measure of groundwater velocity at the candidate PRB location.

NDEP has defined three water-bearing zones (WBZs) that are of interest at the Black Mountain Industrial (BMI) complex: the Shallow Zone, which extends to approximately 90 ft bgs, is unconfined to partially confined, and is considered the "water table aquifer"; the Middle Zone, from approximately 90 to 300 ft bgs; and the Deep Zone, which is defined as the contiguous water-bearing zone that is generally encountered between 300 to 400 ft bgs (NDEP, 2009a). The Shallow Zone will be the focus of the PRB pilot test.

3.3 Groundwater Quality

Within the candidate PRB pilot area, perchlorate concentrations in groundwater samples range from 3 to 18 milligrams per liter (mg/L) (ENVIRON, 2011b, 2012). During the pump test of PC-98R, the following conditions were observed with respect to general groundwater quality parameters (Errol L. Montgomery & Associates, 2000).

- Temperature ranged from 23° to 24° C
- Specific Conductivity ranged from 12,300 to 13,500 microSiemens per centimeter (µSm/cm); and
- pH ranged from 6.90 to 7.70.

Water quality analyses performed by Northgate in 2010 included dissolved metals and anionic species. The results showed a high concentration (1,400 mg/L) of sulfate is present in shallow groundwater. Near the candidate PRB pilot location, nitrate concentrations ranged from 11 to 58 mg/L at MW-K5 and nitrate was detected at 21 mg/L at PC-103 (ENVIRON, 2013). A summary of groundwater indicator parameters and water quality conditions in the candidate location for the PRB pilot is presented in Table 3 and Table 4.

The candidate location for the PRB (as shown in Figure 1, Figure 2a and Figure 2b) is situated adjacent to the bermed and lined ponds of a bird viewing preserve. Water levels and perchlorate concentrations have remained relatively stable in the vicinity of this location since 2011 (ENVIRON, 2013), when the infiltration basins were converted to bird ponds. Water to the ponds is supplied by the treated effluent from the POTW operated by the City of Henderson. A

review of secondary effluent data provided by the City of Henderson shows an average detected concentration of nitrate, nitrite, and BOD at 14.39 mg/L, 0.03 mg/L, and 9 mg/L, respectively, for the month of January 2014 (Analla, 2014). Given their proximity, the ponds of the bird viewing preserve could have an influence on the local hydraulics and the water quality at the candidate location of the PRB pilot and will be evaluated during the pilot test. Groundwater quality, including the presence of electron acceptor species (e.g., oxygen, sulfate, manganese, nitrogen, and nitrate) in the vicinity of the proposed PRB pilot location will be further evaluated as discussed in Section 5 below. Baseline groundwater sampling and analysis is proposed as part of design activities for the PRB pilot and monitoring of groundwater elevations and groundwater quality in the vicinity of the PRB pilot is planned during operation of this system as discussed in Section 6.

4 Technology Overview and Rationale

PRB technology for the removal of perchlorate involves the creation of conditions in the subsurface environment, which are conducive to the growth of biological communities that are able to use perchlorate as an electron acceptor. The conditions required for such a reaction to occur include the presence of a suitable electron donor (or carbon source), appropriate redox potential, and the presence of other agents necessary for biological growth (e.g., trace nutrients). The specific area of the subsurface environment where these conditions are created are referred to as the reactive or treatment zone and constitute the active portion of the PRB. The treatment zones are created in the path of groundwater flow such that perchlorate in groundwater at the Site using an in-situ technology, such as a PRB includes the following challenges:

- Potentially high groundwater velocities;
- Natural competition in the aquifer for electron donor (i.e., electron donor demand);
- Controlling conditions (e.g., redox potential, concentration of electron donor) to limit biofouling; and
- Sustained long-term operation.

The design of the PRB will depend upon various parameters including the characteristics of the formation, the type of amendment (i.e., election donor) to be deployed, and the resulting time necessary to degrade perchlorate to the desired concentration in groundwater (Federal Remediation Technologies Roundtable ([FRTR], 2005). In addition to consideration of the stoichiometry and rate of degradation of perchlorate, dosing of the selected electron donor needs to account for other, abiotic processes that would consume the donor and reduce their bioavailability to degrade perchlorate (Strategic Environmental Research & Development Program (SERDP), 2009).

System design typically requires an estimate of groundwater flow, solute transport, and biodegradation processes that are involved in the application of a bioremediation system. Specifically, these estimates are used to ensure that the treatment system will: 1) biologically degrade perchlorate within the treatment zone, and 2) avoid excess delivery of electron donor. Using electron donor biological decay rates established based on the data obtained in the ISM study and the bench-scale column testing, the projected fate and transport of injected electron donor can be estimated. Thus, electron donor delivery can be optimized to limit downgradient migration (and subsequent secondary impacts such as metals mobilization) while still providing a sufficiently large biological treatment zone, and reducing the potential for biofouling.

4.1 PRB Functional Description

A PRB is an engineered in-situ treatment system and can include active pumping or passive flow through a reactive zone. A PRB is an in-situ, permeable treatment zone designed to intercept and remediate a contaminant plume. The term "barrier" is intended to convey the idea that contaminant migration is impeded; however, the PRB is designed to be more permeable than the surrounding aquifer media so that groundwater can easily flow through the structure without significantly altering groundwater hydrology (Interstate Technology & Regulatory Council [ITRC], 2011).

4.2 PRB Case Study Review

A literature review was performed, to obtain currently available information on the efficacy of pilot tests and full-scale installations of PRBs for treatment of perchlorate and other similar contaminants in groundwater. A summary of the selected PRB case studies reviewed is presented in Table 5. Perchlorate reductions were reported in the range of 86% to 97%. Passive PRBs (i.e., PRBs that utilized a solid substrate placed in situ) were successful in treating perchlorate concentrations from 170,000 micrograms per liter (µg/L) to non-detect levels. PRBs that utilized injection wells for delivery of amendments have been shown to be as effective as passive systems, although performance data for full-scale, long-term operation of such PRBs is limited. Proximal to the candidate PRB location at the Site, an active PRB, which employed groundwater extraction, amendment and re-injection, was operated at the neighboring American Pacific Corporation (AMPAC) site for treatment of perchlorate in groundwater. This system extracted groundwater, mixed in electron donor (sodium benzoate) ex-situ, and reinjected the groundwater downgradient. The active PRB at the AMPAC site successfully reduced perchlorate from influent levels as high as 31,000 µg/L to non-detect levels (AMPAC, 2009). The system operated for approximately six years before it was shut down after the discovery of additional perchlorate source areas. The AMPAC system did not have the capacity to treat the additional perchlorate loading anticipated and was deemed to not be cost effective in treating the additional load associated with the additional sources areas. Subsequently, AMPAC installed a higher-capacity FBR system (AMPAC, 2012).

Although the AMPAC PRB system was successful at reducing perchlorate, the system experienced problems with biofouling. Early on and to improve the infiltration capacity, AMPAC modified the system from a gallery of shallow injection wells to a deep reinjection trench due to biofouling downgradient of the injection site. Biofouling control was also attempted through the injection of several biocides, including peroxide and hypochlorite, at the injection site with varying and inconsistent results. The most effective control measure reported was the addition of an oxygen scavenger, sodium metabisulfite, in amended groundwater prior to re-injection. It was reported that this resulted in lowering of the dissolved oxygen of the injected groundwater from 6 mg/L to less than 1 mg/L (AMPAC, 2011). At the time the PRB system was shut down at the AMPAC site, the flow rate had been reduced to 130 gallon per minute (gpm) from its design flow of 225 gpm due to bio-fouling at the injection location. Operational considerations, such as introduction of oxygen during extraction and reinjection, and potential overdosing of electron donor suggested by the observed reduction of sulfate downgradient of the reinjection wells, likely contributed to the observed biofouling.

The potential for bio-fouling and mobilization of other constituents will be a key consideration during design and operation of the proposed PRB at the NERT Site. In addition to the potential for bio-fouling, the reduction of perchlorate can also result in mobilization of otherwise stable metals (e.g., manganese and iron). Mobilization of iron and manganese was noted in one of the larger pilot studies performed in Rancho Cordova, California. It is noted that manganese was also mobilized during operation of the active PRB at the AMPAC site (AMPAC, 2009, 2011).

5 PRB Pilot Design

As described above, ENVIRON proposes treatability studies at both the bench-scale and pilotscale to gather the necessary information to evaluate the technical feasibility and overall effectiveness of using PRB technology for the sustained treatment of perchlorate in groundwater at the Site. Specifically, ENVIRON intends to:

- 1. Install soil borings and monitoring wells in an area designated for the PRB pilot while also collecting the necessary groundwater and soil cuttings to enable bench-scale testing;
- 2. Conduct in-situ studies and a bench-scale test program to test the efficiency of various electron donors, establish optimal dosing rates, and to develop parameters to enable pilot system design; and
- 3. Complete a final design of the PRB pilot installation at the candidate installation location at the Site.

5.1 Candidate Installation Location

ENVIRON is proposing to locate the PRB pilot in the location previously identified by Shaw and Northgate; approximately 2,000 ft downgradient of the AWF, approximately mid-way between the AWF and SWF. A groundwater potentiometric surface map and a map of perchlorate isoconcentration contours for the proposed location for the PRB pilot are shown in Figure 2a and Figure 2b, respectively. The in-situ PRB pilot will be located to intersect the flow of groundwater in the saturated alluvium overlying the UMCf. The property in the proposed installation location is owned by the City of Henderson. Arrangements for access for installation and monitoring of the PRB pilot will be required prior to installation.

This candidate location has been proposed based on the following:

- The area is far enough from the extraction well fields, such that the injected substrate will not be affected by pumping gradients;
- The area is located within the paleochannels in the UMCf, which appear to influence the direction of groundwater flow from the Site and transport of perchlorate from the Site to the Las Vegas Wash (refer to cross sections on Figures 3 to 5, and Section 3);
- Perchlorate concentrations are elevated (>10 mg/L), making observation of concentration reductions easier and (if successful) effecting a significant mass removal of perchlorate, while not being so high as to prevent effective treatment via the PRB;
- There is sufficient distance downgradient of the test area prior to the Las Vegas Wash to monitor for degradation by-products, dissolution/release of compounds that may adversely affect water quality, and unconsumed substrate; and
- The area is not occupied by existing structures or in close proximity to drainage features/other factors which might influence surface or groundwater flow or access/transportation routes.

5.2 Preliminary Activities

A soil boring, which will be converted to a new permanent monitoring well, will be installed in the area proposed for the PRB pilot. This newly installed monitoring well will be used to collect information necessary to assess local groundwater flow, and to assess the geologic conditions and soil chemistry. The newly installed monitoring well will also provide a location for ISM testing. Prior to drilling activities, land access to the area for installation will be obtained from the City of Henderson. No less than 48 hours prior to the planned drilling activities, Underground Service Alert (USA) will be notified to identify any possible subsurface utilities or piping that may be in the area of the planned installation. Following installation, the newly installed monitoring well will be developed, purged, and sampled. Both the groundwater sampled and the soil cuttings from the well installation will be shipped to the laboratory for analytical and microbial testing. To provide an in-situ measurement of horizontal groundwater flow at the candidate PRB pilot location, single borehole dilution testing will be performed at the newly installed well to measure groundwater flow. These activities are discussed in further detail below.

5.2.1 Soil Boring and Well Installation

A single monitoring well will be drilled in accordance with Nevada Division of Water Resources (NDWR) requirements outlined in Nevada Administrative Code (NAC) Chapter 534, and notices of intent to drill will be submitted to the Division for the new well.

The soil boring for well installation will be conducted using a Mini Sonic drilling rig within the candidate PRB pilot area. Soil cores will be described in the field by an experienced field geologist. Soil borings will be advanced through the alluvium and will be terminated at the contact of the alluvium and Muddy Creek formation. Samples of soil from the saturated zone within the soil boring will be collected for microbial testing to establish a baseline for perchlorate reducing microorganisms and for use in bench-scale column testing.

Upon reaching the target depth at the top of the UMCf, the soil boring will be converted to a permanent monitoring well. The monitoring well will be constructed using 2-inch diameter slotted polyvinyl chloride (PVC) screen from the top of the water table to the top of the UMCf (a length of approximately 25 ft) and 2-inch diameter schedule 40 PVC riser to the ground surface. A filter pack of washed sand will be placed around the well screen to approximately 2 to 3 ft above the top of the screen. A seal consisting of approximately 2 to 3 ft of hydrated bentonite chips will be placed above the filter pack followed by bentonite/cement grout to the surface.

Following installation, the monitoring well will be developed using a submersible pump. Well development will consist of removal of approximately 10 well volumes of groundwater from the monitoring well. Standard Operating Procedures (SOPs) for photoionization detector (PID) screening for environmental sampling, soil sampling, and monitoring well installation and development are provided in Appendix C.

5.2.2 Groundwater Sampling and Analysis

Prior to groundwater sampling, water level measurements will be collected prior to the purging and sampling of the new monitoring well. The depth to water and the total well depth will be measured using an electronic water level meter. The water levels will be determined to the nearest 0.01 of a foot with an accuracy of ± 0.02 ft and the total well depth will be determined to the nearest 0.1 of a foot with an accuracy of ± 0.2 ft.

5.2.3 Single Borehole Dilution Testing

A single borehole dilution test is a relatively simple hydrogeological technique used to determine the volumetric flow rate of groundwater through a borehole. The technique potentially provides a means to obtain hydrogeological properties, without the need to undertake a pumping test, avoiding the logistical difficulties of such testing. This testing method provides a measurement of ambient groundwater velocity and the capability to reveal zones of preferential flow and zones of negligible flow within a single borehole (Pitrak, M., Mares S., and Kobr, M., 2007).

Borehole dilution testing will be performed at the newly installed well in the candidate PRB location. To perform the test, a tracer solution (e.g., bromide, fluoride) of known concentration will be circulated/mixed within the screened interval of the monitoring well. The decline of tracer concentration (i.e., "dilution") with time within the well screen will be monitored directly using a vertical array of tracer specific-ion electrode probes located at known depth intervals. Based on the dilution characteristics observed, the vertical distribution (i.e., heterogeneity) of hydraulic properties and/or flow velocity can be estimated for the formation within the well screen section. The presence of vertical flow within the well screen can also be identified from the probe/depth dilution response pattern. The rate of groundwater flow measured in the borehole dilution test will be used to help establish the duration for placement of ISM Bio-Trap[®]s, to establish the flow rates used in the bench-scale columns, and as a parameter used in the design of the PRB pilot.

5.2.4 Management of Investigation-Derived Wastes

In obtaining soil and groundwater for the bench-scale tests, investigation-derived wastes, including leftover soil cuttings (from drilling of boreholes), groundwater (from purging/development of monitoring wells), and spent personal protective equipment (PPE) will be generated.

Consistent with current management practices and pending waste characterization, waste soil, and spent PPE will be stored in 55-gallon drums, transported to the NERT Site, and staged in a temporary holding area on the Site located away from surface water features and storm drains. The drums will be labeled with a drum identification number, the description of the contents, the date generated, and the point of contact to be reached regarding questions. Based on the results of waste characterization samples, arrangements will be made for disposal.

Purged groundwater will be temporarily stored in suitable containers prior to being transferred to the on-site GWETS for treatment.

5.3 ISM Testing

ISM testing results will be used to generate information useful in the design of the PRB pilot system. The ISM testing will provide information to enable selection of electron donors and dosing rates and to identify the geometry and sizing of the PRB for pilot testing.

The biostimulation ISM units will contain a section of sponge-like cellulosic material saturated with the commercial electron donor solution or solid electron donor material. In addition to the

electron donor and nutrient amendments if needed, the ISM units will contain COC, MICRO, and GEO samplers for evaluation of perchlorate and perchlorate byproducts, electron acceptors, nutrients and microbial populations.

The specific objectives of the proposed ISM testing are:

- 1. Identification of suitable electron donors and supplemental amendments (e.g., nutrients) required for perchlorate reduction.
- 2. Evaluation of the perchlorate degradation rates achievable at the candidate PRB pilot location at the Site.
- 3. Establish degradation and hydraulic parameters required to design a PRB pilot.

The ISM testing will be performed in two stages at the candidate PRB location. The first stage (Stage 1 ISM study) will evaluate a variety of potential electron donors. Based on the results of the Stage 1 ISM study, the most effective electron donor will be evaluated in the subsequent second stage (Stage 2 ISM study) by means of retrieval of ISM Bio-Trap[®]s over time, which will provide an indication of the rate of perchlorate biodegradation in situ. A more detailed description of the proposed two-stage ISM study is provided below.

5.3.1 Stage 1 ISM Testing

The Stage 1 ISM study will include the deployment of Bio-Trap[®]s, to establish the most promising candidate amendment(s) for perchlorate reduction. The traps to be deployed in the Stage 1 study will include one unamended trap allowing for the analysis of monitored natural attenuation parameters. Three additional traps, each amended with a different electron donor substrate will be installed in wells PC-98R, MW-K5 and the newly installed well. The ISMs will be provided by a specialized vendor, Microbial Insights, Inc. (MI), and will be constructed at the PRB location. The following is a list of potential electron donor substrates identified for testing. At least one substrate will be evaluated from each of the three groups described below:

- 1. Soluble electron donors (e.g., lactate, acetate);
- 2. Solid carbon electron donors (e.g., compost and peat, mulch mixed with sand or pea gravel); and
- 3. Proprietary, slow-release electron donor (e.g., Regenesis HRC[®], FMC EHC[®], Duramend[®], EOS Remediation EOS[®]).

The above electron donors were selected based on their ability to be applied to a variety of potential PRB designs (e.g., via direct injection, passive diffusion wells, or within a trenched wall). Each has demonstrated success in similar environments based on review of case studies and published research, and cost-effectiveness in full-scale application (Batelle, 2000, FRTR, 2005, ITRC, 2011).

With respect to soluble donors, acetate was selected as a candidate electron donor to be evaluated because it can be readily metabolized by a variety of microflora and requires relatively low energy to be utilized. Lactate ferments directly to acetate, and has been used in

PRBs at other sites such as the Naval Surface Warfare Center in Indian Head, Maryland (Table 5).

Proprietary electron donors, Regenesis HRC[®], EOS Remediation EOS[®], FMC EHC[®] and Duramend®, have also been identified for testing as these products are designed to provide a slow release that can extend the longevity of the PRB between dosings and can avoid some of the problems with bio-fouling associated with other substrates. Each of these proprietary products has been specifically formulated for use in in-situ anaerobic degradation of halogenated organic compounds, and would be effective at reducing perchlorate. Following approval of this Work Plan, a vendor will be selected to supply one of these proprietary electron donors for testing.

Solid carbon electron donors, hard wood mulch, peat, and compost, have been selected based on their common availability and extended release properties. Each of these solid substrates has advantages and disadvantages. For example, the lignins in mulch are not as readily bioavailable compared to other substrates (compost and peat). However, compost and peat may be less commercially available than mulch and therefore could be more costly. The addition of gravel or sand to these substrates will provide the necessary structure to achieve the desired hydraulic characteristics for flow of groundwater through the PRB. As summarized in Table 5, the use of mulch, compost, and peat as electron donors in PRBs has been demonstrated at sites such as the Naval Weapons Industrial Reserve Plant in McGregor, Texas and Whiteman Air Force Base near Kansas City, Missouri.

Once the amendments and samplers have been added to each ISM unit, the assembled units can be connected to form a single line for in well deployment. A nylon rope or cable is attached to the uppermost ISM unit. A stainless steel weight is added to the bottom-most ISM unit and the assembly is lowered into the monitoring well. The cable is typically attached to an eye bolt in the gripper plug or top of casing. The cable must be long enough to suspend the assembly of ISM units within the screened interval of the saturated zone.

Prior to installation of the ISM units, each well will be purged and the following parameters will be collected approximately every five minutes during the purging process and will be recorded in a field notebook and/or groundwater sampling log forms along with the pumping rate, depth to water, and other observations: pH, conductivity, ORP, and dissolved oxygen (DO). Purging will continue until pH, conductivity and turbidity readings have stabilized over three consecutive readings. The in-line water quality meter will be disconnected prior to sampling. After the wells have been purged and sampled for baseline parameters listed in the below table "Stage 1 ISM Study - Summary of Testing Parameters" following the sampling SOPs of Appendix C. Following receipt of the baseline analytical results, the results will be summarized and submitted to NDEP along with an evaluation of the planned PRB pilot activities included in this Work Plan. Following NDEP's review and acceptance of this evaluation, the activities included in this Work Plan will proceed beginning with deployment of a series of ISM units in each monitoring well.

After the desired minimum incubation period, the Stage 1 ISM units will be retrieved and the samplers removed, appropriately labeled, placed in zippered bags, and shipped overnight on ice

to MI under standard chain-of-custody. The samplers will be analyzed for the following parameters.

Stage 1 ISM Study - Summary of Testing Parameters and Frequency

Parameter (Analytical Method)	Frequency
Perchlorate by ion chromatography-mass spectroscopy- mass spectroscopy (IC-MS/MS) ¹	Baseline groundwater samples and after a minimum of 4 weeks of incubation ³ in-situ.
Nitrate/nitrate (United States Environmental Protection Agency (USEPA) Method 300.0)	
Conductivity (microelectrode)	
Total Kjeldahl Nitrogen (USEPA Method 351.2)	
Orthophosphate (USEPA Method 300.0 or USEPA 365.3)	
Microbial population: perchlorate reducers (qPCR method) and general microbial groups via Phospholipid fatty acid analysis	
Redox indicators plus Chloride	Baseline groundwater samples and after a
- Dissolved oxygen (microelectrode);	minimum of 4 weeks of incubation ³ in-situ.
- Chloride and sulfate (USEPA Method 300.0);	
- Chlorate (USEPA Method 300.1);	
 Sulfide (HACH Method 8131 (USEPA Methylene Blue Method)); 	
 Ferric and ferrous iron (HACH Method 8008 and 8147); and 	
 Methane in headspace (Gas chromatograph – flame ionization (GC-FID)²) 	
Dissolved Metals (Ag, As, B, Ba, Be, Ca, Cd, Cr, Co, Cu, Fe, Hg, K, Mo, Mg, Mn, Na, Ni, Pb, Sb, Se, Tl, Zn), and U) (USEPA Methods 6010/6020/7400/200.8)	Baseline groundwater samples and after a minimum of 4 weeks of incubation ³ in situ.
Quality Assurance/Quality Control (QA/QC)	Duplicates will be run on 5% of the groundwater samples. Typical runs will consist of blanks, daily calibration check samples, and runs of standard reference materials, when available.

Notes:

- ¹ CIO4- concentrations will be measured by sequential ion chromatography-mass spectroscopy-mass spectroscopy (IC-MS/MS). CIO4- will be quantified using a Dionex LC 20 ion chromatography system consisting of GP50 pump, CD25 conductivity detector, AS40 automated sampler and Dionex IonPac AS16 (250 X 2 mm) analytical column. A hydroxide (NaOH) eluent at 0.3 milliliters per minute (mL min-1) is followed by 90% acetonitrile (0.3 mL min-1) as a post-column solvent. To overcome matrix effects, all samples are spiked with CI18O3 or CI18O4 internal standards.
- Kampell, D.H. and S.A. Vandegrift. 1998. Analysis of Dissolved Methane, Ethane, and Ethylene in Ground Water by a Standard Gas Chromatographic Technique. J. of Chromatographic Sci. 36:253-256.
- ³ A minimum incubation period of 4 weeks is anticipated, but may be adjusted based on estimated of ground water velocity from the single borehole dilution testing and the results of initial baseline groundwater sampling.

Based on the results of the initial ISM testing, selection of the amendments for follow-on testing in the Stage 2 ISM study will be determined. It is anticipated that at least two amendments will be selected for the Stage 2 study.

5.3.2 Stage 2 ISM Testing

The Stage 2 study will consist of the deployment of four to five Bio-Trap[®]s, each amended with the selected substrate(s), for deployment in the single new monitoring well. Approximately every 2 to 3 weeks (based on the results of the Stage 1 testing), one of the Bio-Trap[®]s containing each of the candidate amendments will be removed for lab analysis for the parameters listed in the table below. ENVIRON estimates the Stage 2 ISM test to require a total duration of approximately 3 to 5 months. The results of this testing will provide a general rate of degradation under actual in-situ conditions in the field.

Location	Parameter (Analytical Method)	Frequency
MICRO Sample	Microbial Analyses: perchlorate reducers (Microbial Insights, Inc. or similar company/university)	Every 2 to 3 weeks
COC Sampler Perchlorate by IC-MS/MS ¹ Nitrate/nitrate (USEPA Method 300.0) Conductivity (microelectrode)		Every 2 to 3 weeks
GEO or pumped groundwater sample	 Redox indicators plus Chloride Dissolved oxygen (microelectrode), Chloride, nitrite, nitrate, ferrous, ferric iron, sulfate, sulfide (USEPA Method 300.0), Sulfide (HACH Method 8131 (USEPA Methylene Blue Method)) Ferric and ferrous iron (HACH Method 8008 and 8147) Methane in pore water (GC-FID²) 	Every 2 to 3 weeks

ISM Phase 2 Testing - Summary of Testing Parameters and Frequency				
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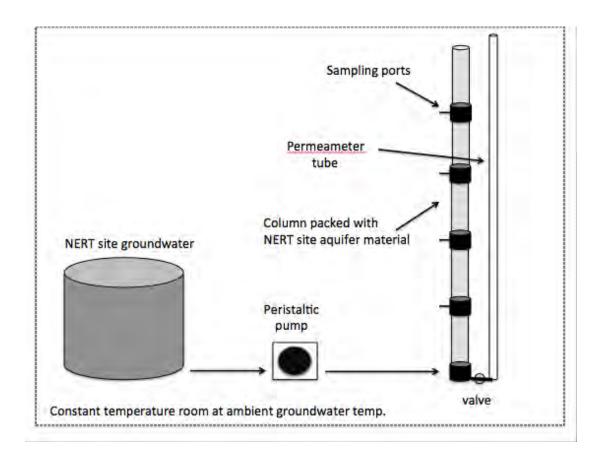
¹ CIO4- concentrations will be measured by sequential ion chromatography-mass spectroscopy-mass spectroscopy (IC-MS/MS). CIO4- will be quantified using a Dionex LC 20 ion chromatography system consisting of GP50 pump, CD25 conductivity detector, AS40 automated sampler and Dionex IonPac AS16 (250 X 2 mm) analytical column. A hydroxide (NaOH) eluent at 0.3 milliliters per minute (mL min-1) is followed by 90% acetonitrile (0.3 mL min-1) as a post-column solvent. To overcome matrix effects, all samples are spiked with Cl18O3 or Cl18O4 internal standards.

Kampell, D.H. and S.A. Vandegrift. 1998. Analysis of Dissolved Methane, Ethane, and Ethylene in Ground Water by a Standard Gas Chromatographic Technique. J. of Chromatographic Sci. 36:253-256.

5.3.3 Bench-Scale Column Testing

Column studies will be performed using the electron donors selected from the results of the Stage 1 ISM study and will be run in parallel with the Stage 2 ISM study activities. The column

study will be used to test the effectiveness of donors in a flow-through mode simulating field conditions of the Site, providing additional information useful in the design of the PRB pilot. Specifically, the column testing will be used to refine the list of potential amendments targeted for the pilot testing. The amendment(s) chosen for the pilot testing will be those that reduce perchlorate but also maintain the hydraulic properties of the formation (minimize biofouling). A schematic diagram of the 1-D column system is shown in the laboratory column setup illustration below.



One column for each candidate amendment selected from the results of the Stage 1 ISM testing, plus one unamended control column will be constructed. Column experiments will be performed in 5-foot long, 2-inch diameter columns with five equally spaced sampling ports located along their lengths. A sample of soil cuttings from within the saturated zone of the soil boring will be submitted to a lab for microbial testing for perchlorate reducing bacteria to establish a baseline for this population. Additionally and at the conclusion of the column testing, a sample of soil from the bottom of each column will be submitted to the lab for microbial testing for perchlorate reducing bacteria.

The columns will be packed with aquifer matrix material from the newly installed monitoring well at the candidate PRB location. A 5-centimeter layer of fine gravel will be placed at the bottom of each column to equalize the distribution of flow through the column. Glass wool will be inserted in the inner side of sampling ports to avoid dead zones and clogging of sampling ports.

Immediately after establishment of the columns, the hydraulic conductivity of the test columns will be assessed by connecting a falling head permeameter to the column. Hydraulic conductivity will be measured using the falling head method and compared to existing data for the Site. A protocol for bench-scale testing prepared by Dr. John Pardue and Dr. W. Andrew Jackson of Louisiana State University (LSU) is provided in Appendix B.

Laboratory Column Set-up

Groundwater collected from the candidate PRB location at the Site will be shipped to the off-site laboratory and introduced through 2 millimeter (mL) stainless steel tubing in up-flow mode. A peristaltic pump with Viton tubing will used to convey water through the column at groundwater velocities representative of conditions at the candidate location for the PRB pilot. The experiment will be set-up in a constant temperature room so that groundwater and the test columns will be maintained at ambient temperatures similar to those present at the candidate PRB location.

The influent concentrations will be monitored three times per week to track changes in perchlorate concentration. Influent samples for all column experiments will be collected at the sampling ports on the delivery side of the pump. Samples from each sample port will be collected every three to four days with a 5 mL pre-rinsed airtight glass syringe fitted with luerlock and injected into 2-mL glass vials. Samples collected will be analyzed for perchlorate, nitrate/nitrite and conductivity. On a weekly basis, additional redox indicators will be measured including dissolved oxygen, ferrous iron, ferric iron, sulfate and sulfide, and methane. Oxidation-reduction characteristics of each sampled zone will be determined from the water chemistry parameter results. Additional samples will be collected from the columns for metals analysis at an external certified laboratory. Column studies will be run for a period of approximately 8 to 12 weeks, with flows through the columns adjusted based on the observed groundwater velocity observed from the single borehole dilution testing, subject to extension if additional information is desired. Following the termination of the studies, the falling head permeameter study will be repeated and the hydraulic conductivity measured again to assess the effect on aquifer hydraulic properties. Declines in conductivity over the duration of testing will provide evidence of conditions that may be conducive to biofouling. If conductivity declines significantly (e.g., greater than 5 to 10 times the initially measured hydraulic conductivity), column materials will be removed and total carbon measured on the aquifer material to determine the amount of biomass accumulated along the flow path.

Analytical Procedures

Major anions (CI⁻, NO³⁻, and SO₄²⁻) will be analyzed by ion chromatography following USEPA Method 300.0. Perchlorate concentrations will be separately measured by sequential ion chromatography-mass spectroscopy-mass spectroscopy (IC-MS/MS). Redox parameters will be measured using standard methods for DO (by microelectrode), nitrite, nitrate, ferrous and ferric iron, sulfate, sulfide (by ion chromatograph), and methane in pore water (by GC-FID). To assess the liberation of metals from the aquifer matrix, samples will also be collected for metals analysis over the course of the column testing. Below is a summary of the testing parameters, analytical methods and frequency for the column testing.

Column Testing - Summary of Testing Parameters and Frequency
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Location	Parameter (Analytical Method)	Frequency
Column influent	Perchlorate by IC-MS/MS ¹	3 times/week for 12 weeks
Sample ports	Perchlorate by IC-MS/MS ¹ , Nitrate/nitrite (USEPA Method 300.0), Conductivity (microelectrode)	Every 3 to 4 days
All Sample Ports	 Redox indicators plus Chloride Dissolved oxygen (microelectrode), Chloride, ferrous and ferric iron, sulfate, sulfide (USEPA Method 300.0), Sulfide (HACH Method 8131 (USEPA Methylene Blue Method)) Ferric and ferrous iron (HACH Method 8008 and 8147) Methane in pore water (GC-FID²) 	Weekly
Column Effluent	Dissolved Metals (Ag, As, B, Ba, Be, Ca, Cd, Cr, Co, Cu, Fe, Hg, K, Mo, Mg, Mn, Na, Ni, Pb, Sb, Se, Ti, Zn, and U) (USEPA Methods 6010/6020/7400/200.8)	Every two weeks
Each Column	Hydraulic conductivity (Falling Head Permeability Test (ASTM D5084-10))	At beginning and after termination of study

Notes:

CIO4- concentrations will be measured by sequential ion chromatography-mass spectroscopy-mass spectroscopy (IC-MS/MS). CIO4- will be quantified using a Dionex LC 20 ion chromatography system consisting of GP50 pump, CD25 conductivity detector, AS40 automated sampler and Dionex IonPac AS16 (250 X 2 mm) analytical column. A hydroxide (NaOH) eluent at 0.3 milliliters per minute (mL min-1) is followed by 90% acetonitrile (0.3 mL min-1) as a post-column solvent. To overcome matrix effects, all samples are spiked with CI18O3 or CI18O4 internal standards.

Kampell, D.H. and S.A. Vandegrift. 1998. Analysis of Dissolved Methane, Ethane, and Ethylene in Ground Water by a Standard Gas Chromatographic Technique. J. of Chromatographic Sci. 36:253-256.

<u>QA/QC</u>

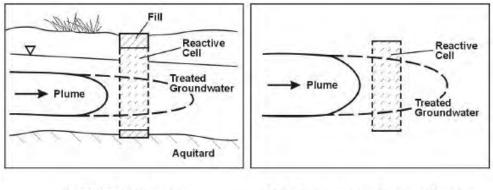
Duplicates will be run on 5% of the samples. Typical runs will consist of blanks, daily calibration check samples, and runs of standard reference materials, when available. Split samples can be provided for analysis upon request.

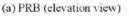
5.3.4 Establishment of Parameters for PRB Pilot Design

The results of the borehole dilution testing for groundwater flow measurement, the Stage 2 ISM study and the bench-scale column testing will be used to establish rate perchlorate reduction and will be applied to reactive-transport models as described below.

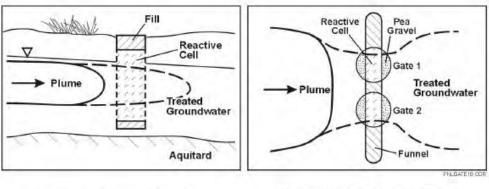
PRB as a treatment technology is a method of mass flux reduction. For the PRB pilot design, it will be necessary to reasonably estimate the mass of reactant that will be needed to treat the mass flux of contaminants. The geologic characteristics in the planned footprint of the area to be treated will also be important in this design. Accordingly, the dosing of amendments, the associated degradation rate and the velocity of groundwater flow through the PRB will be necessary to define for design of the PRB pilot.

The PRB must be able to intercept the contaminant plume without unacceptable contaminant bypass either below or around the barrier. Additionally, effective remediation using a PRB will depend on the availability of appropriate quantities of reactive media and the geochemical and redox conditions to allow for sufficient constituent degradation. The reactive zone must be large (i.e., in thickness and width) enough to allow the degradation. The thickness of the PRB is designed based on the required residence time of the contaminants and the groundwater flow velocity. The residence time must be sufficient to allow for degradation of the target contaminant(s) to reduce the contaminant flux (ITRC, 2011). A schematic of various PRB configurations and flow through a PRB with the associated PRB dimensions are provided in the below illustrations.



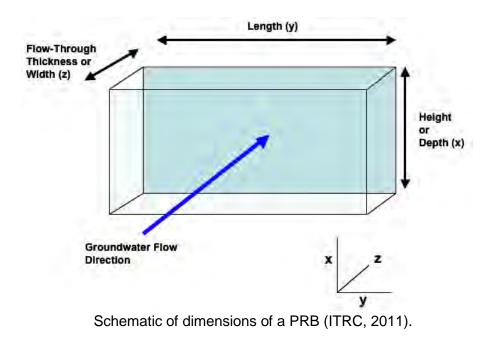


(b) Continuous reactive barrier configuration (plan view)



(c) Hanging barrier configuration for shallow plumes (elevation view) (d) Funnel-and-gate system with two gates (plan view)

Schematic Illustration of Some PRB Configurations (Batelle, 2000).



5.3.5 Reporting

At the conclusion of the Stage 1 ISM study, a letter report summarizing the results of baseline groundwater sampling, the borehole dilution testing and the Stage 1 study along with a recommendation for candidate amendment(s) for follow-on study will be provided to the NDEP. At the conclusion of the bench-scale column testing, a letter report with the results of the Stage 2 ISM study and the bench-scale column testing will be prepared and submitted to the NDEP.

5.3.6 Final Design and Permitting

Utilizing the results of the Stage 2 ISM study and the bench-scale column testing, a Design Report for the Final PRB Pilot will be prepared and submitted to the NDEP. The Design Report will include the detailed plans and specifications for the pilot construction, along with operation and monitoring plans.

Installation of the PRB pilot will require obtaining a General Permit as a Class V Underground Injection Control (UIC) well, if an injectable amendment is selected. Class V UIC wells are non-hazardous wells that inject fluids above the underground source of drinking water (USDW). The injected PRB would qualify for a general permit under the Nevada regulation NAC 445A.891.

In addition to the Class V UIC permit, the PRB pilot will require an application for a UIC General Permit for Short-Term Remediation. UIC General Permits for Short-Term Remediation only allow for a one-time injection of electron donor amendments, and are valid for a period of less than six months. Longer term operation of the PRB pilot may ultimately be required to fully complete the study objectives. In such a case, application for a UIC General Permit for Long-Term Remediation may be necessary at that time.

The permitting process for either Long-Term or Short-Term Remediation Permits requires the submission of the project work plan, a letter of concurrence, UIC Form 200, Notice of Intent (NOI) Form U210, and the respective fees for each permit. General UIC permits are typically issued within 60 days of submission.

Additional permits may be required for construction and will be identified as part of the final design for the PRB pilot.

6 Monitoring

6.1 Preliminary Groundwater Monitoring Plan for PRB Pilot

Groundwater sampling frequency during the PRB pilot test will be established based on the reaction rates observed in the ISM studies and the bench-scale column testing. From the case study review, a potential sampling frequency could be every 2 weeks for the first 60 days, with the frequency decreasing to a monthly sampling rate after the 60-day mark. This sampling frequency was utilized at the Aerojet General Corporation's site in Rancho Cordova, California and was effective in evaluation of perchlorate removal efficiencies in this application. A monthly sampling frequency, as implemented in the Charleston Naval Weapons Station PRB installation, has been shown to provide sufficient data to demonstrate efficacy of the PRB treatment.

A suite of groundwater sampling parameters envisioned in monitoring the performance of the PRB pilot is provided in Table 6. Baseline sampling would be performed for all of the newly installed monitoring wells, existing monitoring wells, and piezometers identified prior to the installation of the PRB pilot system, and would be sampled quarterly thereafter during operation of the PRB. Based on the results observed, it may be possible to reduce or eliminate certain parameters from the monitoring program. Performance monitoring would be performed based on results obtained and Site conditions. It is currently anticipated to be performed after the installation and commencement of operation of the PRB and monthly thereafter during PRB operations.

6.2 Monitoring Well Locations

A conceptual layout of the monitoring wells and piezometers for the PRB pilot system installation is illustrated on Figure 6. A staggered well layout was selected to provide for monitoring of the groundwater conditions both laterally and downgradient of the PRB pilot system. The illustrated spacing of the monitoring wells was based on an assumed hydraulic conductivity of approximately 35 ft/day and the results of the Northgate bench-scale study that indicated successful perchlorate reductions within 14 days. Existing wells (PC-98R and MW-K5) will also be used to provide information on upgradient groundwater quality and elevations. A monitoring well located within the PRB itself is included to provide information on the geochemistry within the wall and to provide a means to observe signs of potential biofouling. Piezometers are included to monitor for changes in groundwater elevations as impacts to groundwater flow, or reductions in hydraulic conductivity that could signal biofouling of the PRB pilot system.

7 Schedule

A preliminary schedule for implementing the activities presented in this Work Plan is provided on Figure 7. The duration of the ISM studies is based on experience and the time necessary for acclimation of the microflora and for adjustments in dosing rates. Based on the results of the ISM studies and the bench-scale column tests, the design for the PRB pilot would be finalized, along with a schedule for installation and associated plans (e.g., final operations and monitoring plans). A preliminary schedule for the PRB treatability study is presented in Figure 7. The timing of events presented in Figure 7 is based on months from the date of submission of this Work Plan to the NDEP. Following receipt of NDEP approval of this Work Plan, an updated schedule that provides specific dates will be submitted to NDEP within the RI Cost Documentation to be submitted in July. The time frame presented may need to be adjusted based on the PRB pilot design. If the schedule must be modified, NDEP will be notified of the anticipated schedule changes.

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Tables

TABLE 1Summary of Work Performed by Others

Date of Study	Type of Study	Performed by	Details of Testing/Observations	Conclusions/Results
12/19/2000	Hydrogeologic	Errol L. Montgomery and Associates Inc.	In 2000, Errol L. Montgomery and Associates Inc. performed an assessment on the siteSite titled "Analysis of Rate of Groundwater Movement Based on Results of Tracer and Hydraulic Tests Conducted between Pittman Lateral and Seep Area, Henderson, Nevada (Errol L. Montgomery & Associates, 2000). This assessment was undertaken prior to establishment of the existing GWETS system, therefore the conclusions of the study may not be entirely representative of current hydrogeological conditions. The assessment was undertaken in order to determine the rate of groundwater flow across the Site area which in turn could be used to estimate the rate of perchlorate transport within groundwater across the Site. The assessment comprised three study areas one of which, Area B (near monitoring well MW-K5) being in the area of the proposed PRB field scale trial. The assessment comprised tracer testing using bromide and deionized water and hydraulic tests.	The assessment determined the following with respect to A • Rate of groundwater movement was in the range of 30 to • Aquifer thickness was 25 ft, transmissivity was 55,000 gal conductivity was 2,200 gpd/ft2. The report also noted that the lower parts of the aquifer (i.e grained sediments which appear to facilitate more rapid gro results of a pump test, performed at monitoring well PC-981 wereas reported. The pump test ran for 29.9 hours and the gallons per minute. The results of the pump test were: • Transmissivity was circa 60,000 gpd/ft; • Hydraulic conductivity was estimated at 2,400 gpd/ft2; and • Storativity was approximately 0.08 (Errol L. Montgomery 8)
1/18/2001	Seep Groundwater Characterization	Kerr-McGee Chemical, LLC	 Work was undertaken to provide supplementary information in the design of the GWETS system. The specific objectives of the assessment were to: Determine the hydrogeologic regime in the area between the Pittman lateral and the Seep; Determine the representative perchlorate concentration in the saturated thickness of the alluvial aquifer near the Seep; Determine if any additional pathways exist along the Las Vegas Wash for other significant perchlorate contribution; Determine the rate of movement and the residence time for perchlorate and groundwater between the Pittman lateral and the Seep; and Determine potential groundwater pumping strategies. 	The results of the investigation indicated: • The BMI Lower Ponds area (encompassing the Seep) wad discharge containing significant perchlorate concentrations • In the Lower Ponds area, the main north/northeast trendir second poorly defined paleochannel entering the area from • In the Lower Ponds area, where the two paleochannels contained perchlorate >10 milligrams per lite 2,200 feetft; • The COH-RIB facility contributed significant amounts of trendom periods of time and directly contributed to daylighticate and to wide fluctuations in both the flow volume and perchlorate be averaged 35 ft/day and the residence time was approximated to the second se
2/14/2010	Work Plan for PRB Pilot Testing	Shaw Environmental, Inc.	A Work Plan was prepared to undertake a field-scale trial of a PRB comprising the injection of slow release, edible oil organic substrate (EOS®598) into the saturated alluvium overlying the Muddy Creek Formation. The PRB would be formed using a series of fixed point injection locations installed to a depth of 40 ft bgs.	In an NDEP letter to Shaw dated April 15, 2010), the Depa assessments had been carried out in the proposed PRB ar gradient tracer tests and injected/pump-back tracer tests (k and Associates, 2000) and that these should be considered PRB. Shaw did not progress to actually undertaking the fie
10/25/2010	Emulsion Retention Testing and Bench-Scale Jar Testing	Northgate Environmental Management, Inc.	Northgate produced a Work Plan to conduct an in-situ PRB pilot test for perchlorate impacted groundwater at the Site. The scope of the Work Plan was to perform both laboratory bench-scale testing and a field-scale pilot test. The overall objective of the proposed pilot test was to examine the feasibility of the use of emulsified oil substrate injected into the subsurface as a PRB to degrade perchlorate in the groundwater; the rationale being that PRBs using edible oil-based electron donor substrates have been shown to be effective in remediation of perchlorate contaminated groundwater. Northgate referenced the Provisional Standard for perchlorate set by NDEP of 18 μg/L as a target for groundwater perchlorate concentrations following treatment by the proposed PRB, the distance from the PRB at which this would be achieved would be dependent upon the results of the field-scale pilot testing. The tests were conducted with the following specific objectives: • To determine the effective retention of EOS® 598B42 and lecithin-modified EOS® 598B42 emulsified oil onto Site-specific soils; • To chemically analyze the Site soil and groundwater to determine concentrations of metals and competing electron acceptors; • To perform leachability tests on the Site derived soil using deionized water to determine a baseline for adsorbed metals stability; • To establish the change in oxidation-reduction potential by adding EOS® 598B42 electron donor substrate to the Site derived soil and groundwater in the presence of indigenous bacteria, perchlorate and competing electron acceptors; • To determine the rate of perchlorate reduction in the test reactors; and • To determine the effect of oxidation-reduction potential on metals stability.	Northgate drilled one borehole in the location of the propos recovered both soil cuttings and groundwater from the oper tests. The untreated groundwater was analyzed for metals to contain 25.7 mg/L perchlorate. Northgate concluded that a maximum effective oil retention 0.06 g/g for lecithin –modified EOS® 598B42, which excee was required to achieve the pilot test objectives. Batch tests were then undertaken to assess the behavior o groundwater was exposed to EOS® 598B42 in the presence acceptors. Northgate concluded that the addition of EOS® anaerobically biodegrade perchlorate without significant mod 4 milliliter (mL) of EOS® 598B42 per liter of groundwater le laboratory reporting limit within 14 days. Northgate asserte would not be expected to occur in the field due to a constant chlorate and perchlorate entering the PRB, a condition that testing that was performed. Northgate did not progress to a

o Area B: to 45 ft(ft)/day; and gallons per day (gpd)/ft and hydraulic

(i.e.i.e., the alluvium) comprise coarser groundwater movement. Specifically, the 98R and within the candidate PRB pilot area, the average pumping rate was circa 52

and ry & Associates, 2000).

was the only identified groundwater ns entering the Las Vegas Wash; ding alluvial paleochannel coalesces with a om the southwest;

coalesce, the entire saturated interval of the liter (mg/L) over a width of approximately

f treated wastewater at random times for hting of groundwater in the Lower Ponds d perchlorate content of the Seep; and between the Pittman Lateral and the Seep nately six months.

partment commented that other, pertinent area employing aquifer tests, natural (Kerr-McGee, 2001 and Errol Montgomery red in justifying the proposed location of the field scale trial.

osed PBR (I-2) to a depth of 40 ft bgs and pen borehole for use in the bench scale Is and perchlorate concentrations and found

ion ratio of 0.02 g/g for EOS® 598B42 and eeded the minimum retention of 0.001 g/g,

or of metals when the soil, saturated with ence of perchlorate and competing electron S® 598B42 stimulated indigenous bacteria to mobilization of arsenic. The additon of 2 to r led to removal of perchlorate to below the erted that the evolution of dissolved arsenic stant flux of dissolved oxygen, nitrate, hat was not possible to be created in the jar to a field-scale trial of a PRB.



TABLE 2Well Construction Details for Existing WellsCandidate FleId-Scale PRB Pilot Test Location

Monitoring Well ID	Well Diameter (inches)	Drilling Method	Well Material	Screened Zone	Date Completed	Total Depth (feet bgs)	Screen Depth (feet bgs)
MW-K5	2	Unknown	Unknown	28.5-43.5	4/2/1998	43.5	28.5
PC-100	2	Hollow Stem Auger	PVC	8.5-38.5	5/18/2000	39	8.5
PC-100R ¹	2	Unknown	PVC	15-40	8/16/2000	40.5	15
PC-103	2	Unknown	PVC	9-29	2/3/2001	29.5	9
PC-2	2	Hollow Stem Auger	PVC	16.7-31.7	3/13/1998	32	16.7
PC-53	2	Hollow Stem Auger	PVC	13-32.5	5/4/1998	33	13
PC-98	4	Hollow Stem Auger	PVC	13.5-33	5/17/2000	33.5	13.5
PC-98R	4	Unknown	PVC	20-35	8/8/2000	40.5	20
PC-1	2	Hollow Stem Auger	PVC	14.7-29.7	3/24/1998	32	29.7
PC-4	2	Hollow Stem Auger	PVC	17.7-42.7	3/24/1998	45	42.7

Notes:

1.) Well PC-100R was abandoned in June 2003.



TABLE 3Summary of Ground Water Indicator ParametersVicinity of Candidate Field-Scale PRB Test Location

	Well	Water Level	Chlorate ¹	<u>Nitrate</u> 1	Sulfate ²	<u>DO</u> ²	<u>ORP</u> ²	<u>рН</u> ^{1,3}	<u>TOC</u> ²	Alkalinity ²	Perchlorate ¹	<u>TDS</u> ¹	<u>Cr Total¹</u>
		<u>(ft msl)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>	<u>(mV)</u>	<u>(s.u.)</u>	<u>(mg/L)</u>	(mg/L as CaCO ₃)	<u>(mg/L)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>
NO	MW-K5	1566.53 to 1569.84	78 to 94	11 to 58				7.20 to 7.57			19 to 27	6300 to 7600	0.024 to 0.053
DCATI	PC-103	1574.93 to 1577.31	1.9	21				7.29 to 7.71			14 to 23	4200 to 5700	0.00096 J to 0.0027 J
NEAR PRB LOCATION	PC-98R	1568.34 to 1571.45				5.3	-35.2	7.12 to 7.55			20 to 27	6000 to 6900	0.012 to 0.021 J
AR P	PC-53	1565.41 to 1569.09						7.42 to 7.69			1.0 to 3.7	4200 to 5700	0.039 to 0.064
Ž	PC-2	1568.59 to 1568.91	22 to 24	11 to 64				7.63 to 7.70			4.4 to 4.5	5500 to 5700	0.012 to 0.077
ax –	ARP-4A	1585.20 to 1587.12						7.32 to 7.69			26 to 32	4300 to 5400	.0041 to 0.012
OF P VTION	ARP-5A	1582.78 to 1584.25						7.52 to 7.71			11 to 19	4900 to 6300	0.031 to 0.046
SOUTH OF PRB LOCATION	ARP-6B	1581.51 to 1584.49				5.2	-240.4	7.28 to 7.53			22 to 56	8100 to 10000	0.22 to 0.34
SC	ARP-7	1581.50 to 1583.80						7.23 to 7.48			4.2 to 6.8	5700 to 7000	0.031 to 0.042
NO	PC-56	1552.71 to 1555.81				0.48	-43	7.62 to 8.14			14 to 23	4600 to 6400	0.0022 J to 0.0059 J
DCATI	PC-58	1551.36 to 1554.34						7.82 to 8.24			1.3 to 2.9	3100 to 8600	0.027 to 0.041
NORTH OF PRB LOCATION	PC-59	1554.06 to 1556.42						7.69 to 8.20			3.8 to 6.2	2900 to 3600	ND to 0.0022 J
	PC-60	1553.05 to 1556.17						7.78 to 8.28			22.6 to 4.6	2400 to 2900	ND to 0.00063 J
	PC-62	1554.42 to 1556.91						7.67 to 8.22			0.081 to 2.4	1800 to 3000	ND to 0.0022 J
ON N	PC-68	1554.18 to 1557.42						7.66 to 8.13			ND to 0.057	1800 to 2300	ND to 0.0013 J



TABLE 3Summary of Ground Water Indicator ParametersVicinity of Candidate Field-Scale PRB Test Location

		<u>Well</u>	Water Level	Chlorate ¹	Nitrate ¹	Sulfate ²	<u>DO</u> ²	<u>ORP</u> ²	<u>рН</u> ^{1,3}	TOC ²	<u>Alkalinity²</u>	Perchlorate ¹	<u>TDS</u> ¹	<u>Cr Total¹</u>
	<u>vven</u>	<u>(ft msl)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>	<u>(mV)</u>	<u>(s.u.)</u>	<u>(mg/L)</u>	<u>(mg/L as</u> <u>CaCO₃)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>	<u>(mg/L)</u>	
니는 님	RIER ALL	M-98	dry			1100								
	BARI WA	M-100	dry			3520 to 3530				50 U	5 U			

Notes:

- 1 Chlorate, Nitrate, pH, Perchlorate, Total Dissolved Solids, and Total Chromium data are from the ENVIRON Annual Remedial Performance Report for Chromium and Perchlorate, July 2012 - June 2013, dated August 30, 2013.
- 2 Sulfate, Dissolved Oxygen, Oxidation Reduction Potential, Total Organic Carbon, and Alkalinity data are from the NERT Analytical Database and are from 2006 to 2010.
- 3 Note that laboratory pH results included in this table have the qualifier "HF", indicating that pH is a field parameter with a hold time of 15 minutes.
- Where applicable, a range of concentrations are given.
- J Result is less than the RL but greater than or equal to the MDL and the concentration is an approximate value.
- U Analyte not detected above the detection limit.



TABLE 4Summary of Ground Water Quality Results

Parameter	Units	Results
Dissolved Metals		
Antimony	mg/L	< 0.005
Arsenic (tot)	mg/L	0.034
Arsenc (recoverable)	mg/L	0.0378
Arsenic (III)	mg/L	< 0.000074
Arsenic (V)	mg/L	0.0319
Beryllium	mg/L	< 0.004
Cadmium	mg/L	< 0.005
Chromium (total)	mg/L	< 0.005
Chromium (VI)	mg/L	< 0.001
Copper	mg/L	< 0.01
Iron (tot)	mg/L	1.6
Iron (II)	mg/L	0.11
Lead	mg/L	< 0.005
Mercury	mg/L	< 0.001
Nickel	mg/L	0.014
Selenium	mg/L	0.01
Silver	mg/L	< 0.005
Thallium	mg/L	< 0.002
Zinc	mg/L	< 0.1
Anionic Species and	Other Parameter	S
Chloride	mg/L	2200
Chlorate	mg/L	28
Perchlorate	mg/L	25.7
Nitrate	mg/L	8.1
Sulfate	mg/L	1400
Sulfide	mg/L	< 0.1
DO	mg/L	8.5
DOC	mg/L	4.4
ORP	mg/L	146
рН	mg/L	7.42

Source:

Northgate. 2011. Bench-Scale Experiments in Support of an In Situ Permeable Reactive Barrier Pilot Test: Effective Oil Retention, Biological Reduction of Perchlorate, and Metals Mobilization Using Site-Specific Soils and Groundwater. March 28.



TABLE 5Summary of Selected PRB Case Studies

Technology	Hydraulic Details	Location	Contaminants	Pilot/Full Scale	Cost	Performance	Longevity
Nano Scale Zero Valent Iron injection	Deep injection	Goodyear, AZ	TCE, perchlorate	In field pilot test			N/A
In situ horizontal flow treatment barrier wells using citric acid for	from Deep Aquifer Region to shallower aquifer region	CA	impacted groundwater (co-	scale test		an average 95% from start to Day 275. Shallow well perchlorate concentrations went from 2230 µg/L to 90 µg/L. Deep well perchlorate concentrations decreased from 3722 µg/L to 1780 µg/L. Mn and Fe were not mobilized. Showed rebound of perchlorate between phased	Long term operation is feasible
	Hydraulic conductivity of 15 ft/day		There were concerns about mobilizing Mn and Fe.		O&M for 30 yrs: \$784,944		
	Injections occurred from 46-61 ft bls for upper section, and 80-100 ft bls for lower section				Long term monitoring: \$271, 342		
Emulsified Oil Substrate (EOS) Biobarrier	Shallow injections (15 bgs).	Elkton, MD			estimated at		Effectiveness of barrier lasted 2.5 to 3.5 years
	50 feet wide GW flow velocity = 100 ft/year, Ground permeability = 29 ft/day						
Biobarrier (mushroom compost, pine wood chips, soybean oil, and 1" crushed limestone) with injected emulsified oil substrate (EOS) solution	Shallow	McGregor, TX					N/A
	Nano Scale Zero Valent Iron injection In situ horizontal flow treatment barrier wells using citric acid for electron donor to stimulate bioremediation Emulsified Oil Substrate (EOS) Biobarrier Biobarrier Biobarrier Biobarrier injected emulsified oil substrate (EOS)	Nano Scale Zero Valent Iron injectionDeep injectionIn situ horizontal flow treatment barrier wells using citric acid for electron donor to stimulate bioremediationUsed recirculation of water from Deep Aquifer Region to shallower aquifer region back to Deep.Hydraulic to shallower aquifer region back to Deep.Hydraulic conductivity of 15 ft/dayEmulsified Oil Substrate (EOS) BiobarrierShallow injections (15 bgs).Biobarrier (mushroom compost, pine wood chips, soybean oil, and 1" crushed limestone) with injected emulsified oil substrate (EOS)Shallow	Nano Scale Zero Valent Iron injectionDeep injectionGoodyear, AZIn situ horizontal flow treatment barrier wells using citric acid for electron donor to stimulate bioremediationUsed recirculation of water to shallower aquifer Region back to Deep.Rancho Cordova, CAHydraulic conductivity of 15 ft/dayHydraulic conductivity of 15 ft/dayInjections occurred from 46-61 ft bls for upper section, and 80-100 ft bls for lower sectionElkton, MDEmulsified Oil Substrate (EOS) BiobarrierShallow injections (15 bgs).Elkton, MDBiobarrier (mushroom compost, pine wood chips, soybean oil, and 1" crushed limestone) with injected emulsified oil substrate (EOS)ShallowBiobarrier (EOS) Biobarrier (mushroom compost, pine wood chips, soybean oil, and 1" crushed limestone) with injected emulsified oil substrate (EOS)Shallow	Nano Scale Zero Valent Iron injection Deep injection Goodyear, AZ TCE, perchlorate In situ horizontal flow treatment barrier wells using citric acid for electron donor to stimulate bioremediation Used recirculation of water from Deep Aquifer Region back to Deep. Rancho Cordova, CA Perchlorate impacted groundwater (co- contaminants include nitrate and TCE). bioremediation Hydraulic conductivity of 15 ft/day There were concerns about mobilizing Mn and Fe. Emulsified Oil Substrate (EOS) Biobarrier Shallow injections (15 bgs). Elkton, MD Perchlorate and chlorinated solvents S0 feet wide GW flow velocity = 100 ft/year, Ground permeability = 29 ft/day Sol feet wide GW flow velocity = 100 ft/year, Ground permeability = 29 ft/day McGregor, TX Perchlorate contaminated ground water	Nano Scale Zero Valent Iron injection Deep injection Goodyear, AZ TCE, perchlorate In field pilot test In situ horizontal flow Valent Iron injection Used recirculation of water Rancho Cordova, Rancho Cordova, Perchlorate In field pilot / impacted In situ horizontal flow Valent Iron Deep Aquifer Region lectron donor to stimulate CA Perchlorate In field pilot / impacted bioremediation back to Deep. CA CA For any and the provide test bioremediation Hydraulic conductivity of 15 ft/day There were concerns about mobilizing Mn and Fe. There were concerns about mobilizing Mn and Fe. Emulsified Oil Substrate (EOS) Shallow injections (15 bgs). Elkton, MD Perchlorate and chlorinated solvents In field pilot S0 feet wide GW flow velocity = 100 ft/day So feet wide GW flow velocity = 100 ft/day McGregor, TX Perchlorate contaminated ground water Full scale	Namo Scale Zero Valent Iron injection Deep injection Goodyear, AZ TCE, perchlorate In field pilot test N/A In situ horizontal flow treatment barrier wells imulate bioremediation Used recirculation of water Rancho Cordova, impacted to shallower aquifer region back to Deep. Perchlorate impacted to shallower aquifer region back to Deep. In field pilot / demonstration scale test Capital: \$403,205 Hydraulic conductivity of 15 ft/day Hydraulic conductivity of 15 ft/day There were concerns about mobilizing Mn and Fe. O&M for 30 yrs: \$784,944 Emulsified Oil Substrate (EOS) Biobarrier Injections occurred from 46-61 ft bis for upper section, and 80-100 ft bis for lower section Elkton, MD Perchlorate and chlorinated solvents In field pilot \$242 Emulsified Oil Substrate (EOS) Biobarrier S0 feet wide GW flow velocity = 100 ft/gay, Ground permeability = 29 ft/day Elkton, MD Perchlorate contaminated solvents Full scale \$200/ft^2 per linear foot, or less than \$15 per linear foot, or less than \$15 per linear foot	Nano Scale Zero Valent fron injection Deep injection Goodyear, AZ TCE, perchlorate in field pilot / inst un-orizontal flow Experienced TCE rebound; hydrogen concentrations increased In situ horizontal flow Used recirculation of water reatment barrier wells from Deep Aquifer Region electron donor to stimulate bioremediation In field pilot / impacted an average 95% from start to Day 275. Stallow well perchlorate concentrations decreased an average 95% from start to Day 275. Stallow well perchlorate concentrations decreased on average 95% from start to Day 275. Stallow well perchlorate concentrations decreased on average 95% from start to Day 275. Stallow well perchlorate concentrations decreased from 3722 up/L to 7700 up/L. Mn and Fe were not mobilized. Showed rebound of perchlorate between phased operations. Hydraulic conductivity of 15 ft/day There were concerns about mobilizing Mn and Fe. O&M for 30 yrs: \$784,944 O&M for 30 yrs: \$784,944 Emulsified Oil Substrate (EOS) Biobarrier Shallow injections (15 Biobarrier Elkton, MD Perchlorate and chlorinated solvents In field pilot start deviced after monitoring: \$271. 342 A 200 ft PRB Perchlorate concentrations deviced stantate conductivity reduced aprecipient and perchlorate noncentration deviced potentially due to biomass growth. Biobarrier S0 feet wide GW flow velocity = 100 (thyser, Ground permeability = 29 (triags water McGregor, TX Perchlorate contaminated ground water Full scale contaminated ground water S0000 or \$19rt. inear foot Reduced perchlo



TABLE 5Summary of Selected PRB Case Studies

Site Name	Technology	Hydraulic Details	Location	Contaminants	Pilot/Full Scale	Cost	Performance	Longevity
	Biobarrier (organic mulch and clean sand)	Shallow (10 to 20 ft deep)	MO	CVOCs, primarily TCE (groundwater contaminants)	Full Scale	Total \$74,000 or \$275/linear foot, less than \$20 per vertical foot	Monitoring shows CVOC degradation within the biobarrier, CVOC concentrations in downgradient wells are 88% lower than in upgradient wells	Continued to show effective treatment after 2 years of operation
Confidential Industrial Site research funded by ESTCP	Emulsified oil (EOS) injected to form a Permeable Reactive Barrier (PRB)	Shallow (10 ft deep, 10 ft wide, 50 ft long). Shallow hydraulic gradient of 0.003 ft/ft, hydraulic conductivity averaged between 22 to 40 ft/day. Assuming 30% porosity, ground water velocity was approximately 80 ft/year.		Perchlorate and TCE plume	Pilot	the site estimated at \$38,000, or	Dissolved iron increased from non- detect to a maximum of 78 mg/L, manganese also increased. Perchlorate rebound experienced 4 months after injection, but concentrations continued to decrease for 7 more months.	At least 3.5 years (monitoring ended after 3.5 years)
		Average GW velocity in specific test area calculated to be 400 ft/year.				30 yr life cycle cost estimated at \$161,400	Average removal efficiency of perchlorate was 97% (reduced from 10,000 µg/L to <4 µg/L) 10' downgradient of injection wells.	
	28 months after initial injection, a buffered EOS was injected.	Shallow (10 ft deep), used a small grid configuration Aquifer between 0.5 ft and 6 ft bgs Hydraulic conductivity of surficial aquifer 1 to 10 ft/day	S.C.	TCE	Pilot	\$428/ cu yd for a recirculation design	Ground water was oxidative, determined this is not optimal for biodegradation. TCE was reduced by 76 to 86% lower	Initial injection treatment continued to work for at least 28 months, second injection treatment prolonged treatment out to 3.5 years (end of monitoring)
Warfare Center	Recirculation treatment using sodium lactate as electron donor, with a sodium bicarbonate buffer	Average hydraulic conductivity of 5.2 ft/day and 2.7 ft/day in Mainland	Indian Head, MD	Perchlorate	Pilot	30 year total cost \$2,243,853 including monitoring. First year cost \$311,837	Reduced from 170,000 μg/L to below detection (5 μg/L)	Biobarrier can be continually replenished by sodium lactate injection; study lasted 20 weeks



TABLE 5Summary of Selected PRB Case Studies

Site Name	Technology	Hydraulic Details	Location	Contaminants	Pilot/Full Scale	Cost	Performance	Longevity
Confidential Industrial Site		PRB installed to a depth of 25 ft bgs to target the permeable gravel zone at that depth. Ground water flow velocity of 25 to 51 ft/year.		Perchlorate (impacted soil and groundwater)	Full scale	trenching, cost \$185/linear foot	ft downgradient of the PRB. Ferrous	
Grain Silo Facility Kansas	EHC injection from Adventus	Ground water table encountered at 23 ft bgs Ground water velocity averages 1.8 ft/day	Kansas	Carbon tetrachloride and its catabolites	Pilot		Carbon tetrachloride was reduced by up to 99.5%; initial concentration was 1,000 ppb, final concentration measured was 5 ppb	Documented operation of over 4 years with continuous removal of carbon tetrachloride at or over 94%

References:

- Environmental Protection Agency November 2006, Technology News and Trends.
- Environmental Alliance December 2006, Application of Mulch Biowall for Anaerobic Treatment of Perchlorate in Shallow Groundwater.
- Shaw Environmental July 2009, In Situ Bioremediation of Perchlorate in Groundwater.
- CH2MHill February 2010, The Evolution of a Field Application of nano Scale Zero Valent Iron (nZVI) in a Deep Low Permeability Aquifer.
- Solutions-IES February 2010, Edible Oil Barriers for Treatment of Chlorinated Solvent and Perchlorate-Contaminated Groundwater.
- Solutions-IES July 2010, Evaluation of Potential for Monitored Natural Attenuation of Perchlorate in Groundwater (Indian Head).



TABLE 6 Analytical Parameters PRB Monitoring - PRB Pilot

Parameter	Method
Temperature, pH, Conductivity, DO and ORP	Portable field instrument
Groundwater elevation	Portable field instrument
Turbidity	USEPA Method 180.1
Total Organic Carbon (TOC)	USEPA Method 415.1
Dissolved Organic Carbon (DOC)	USEPA Method 415.1
Total Nitrogen	USEPA Method 351.1
Total Phosphorous	USEPA Method 365.1
Alkalinity	USEPA Method 310.2
Hardness	USEPA Method 130.1
Total Dissolved Solids (TDS)	USEPA Method 160.1
Perchlorate	USEPA Method 314
Chlorate / Chlorite	USEPA Method 300.1
Chloride	USEPA Method 300.0
Dissolved Metals (Ag, As, B, Ba, Be,Ca, Cd, Cr, Co, Cu, Fe, Hg, K, Mo,	
Mg, Mn, Na, Ni, Pb, Sb, Se, TI, Zn), and U)	USEPA Methods 6010/6020/7400/200.8
Ferrous and Ferric Iron	HACH Method 8008 and 8147
Nitrate / Nitrite	USEPA Method 300.0
Sulfate	USEPA Method 300.0
	HACH Method 8131 (USEPA Methylene
Sulfide	Blue Method
Methane	

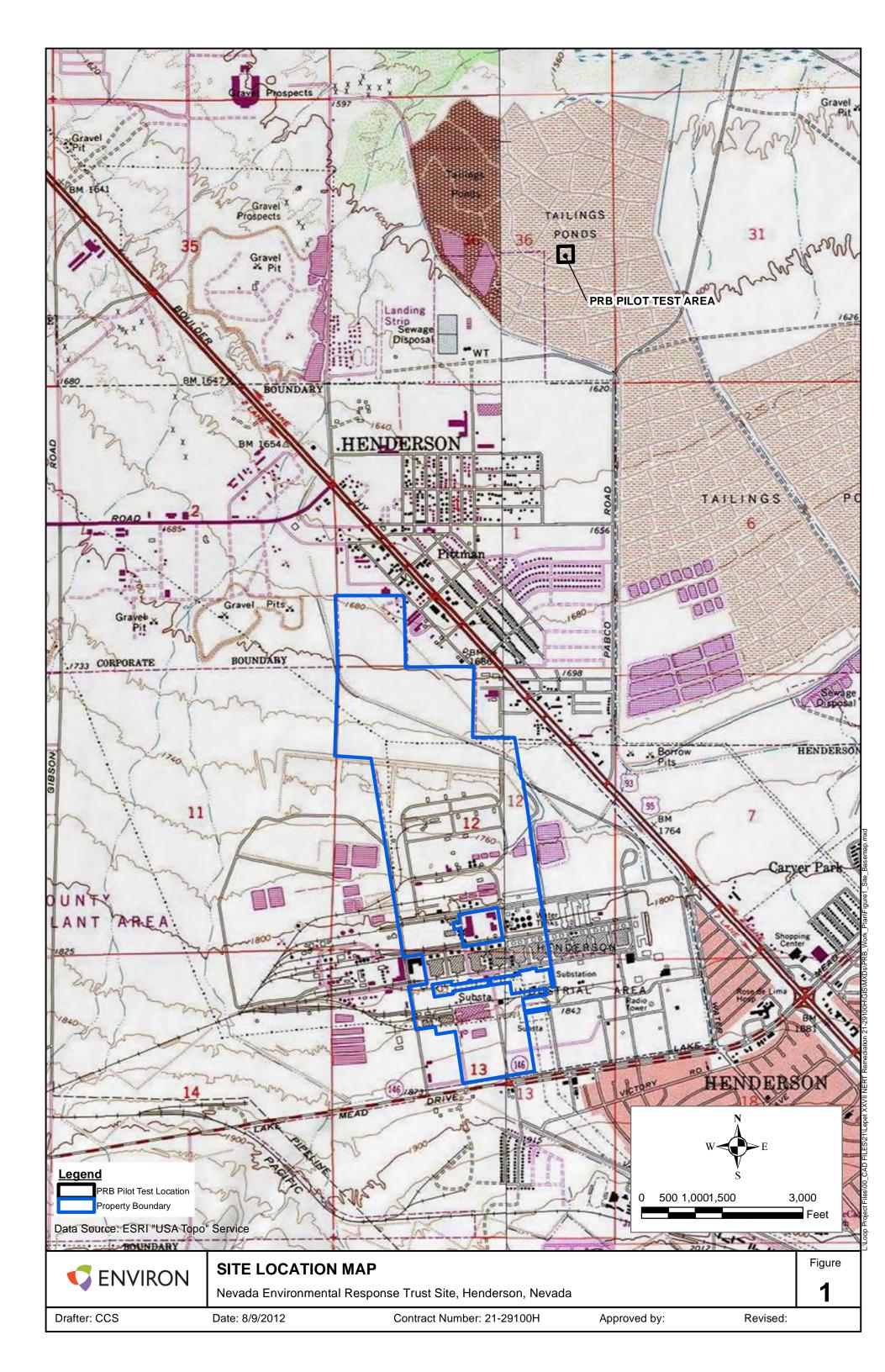
Baseline and Quarterly Sampling Parameters

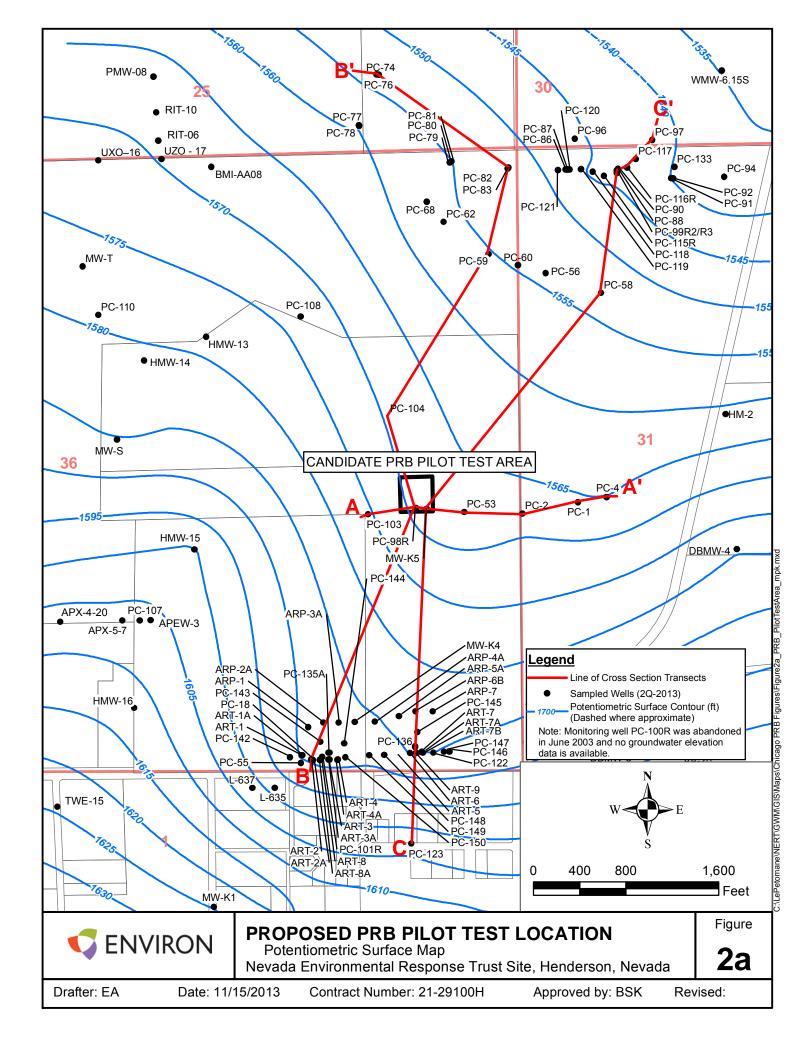
Parameters for Performance Monitoring

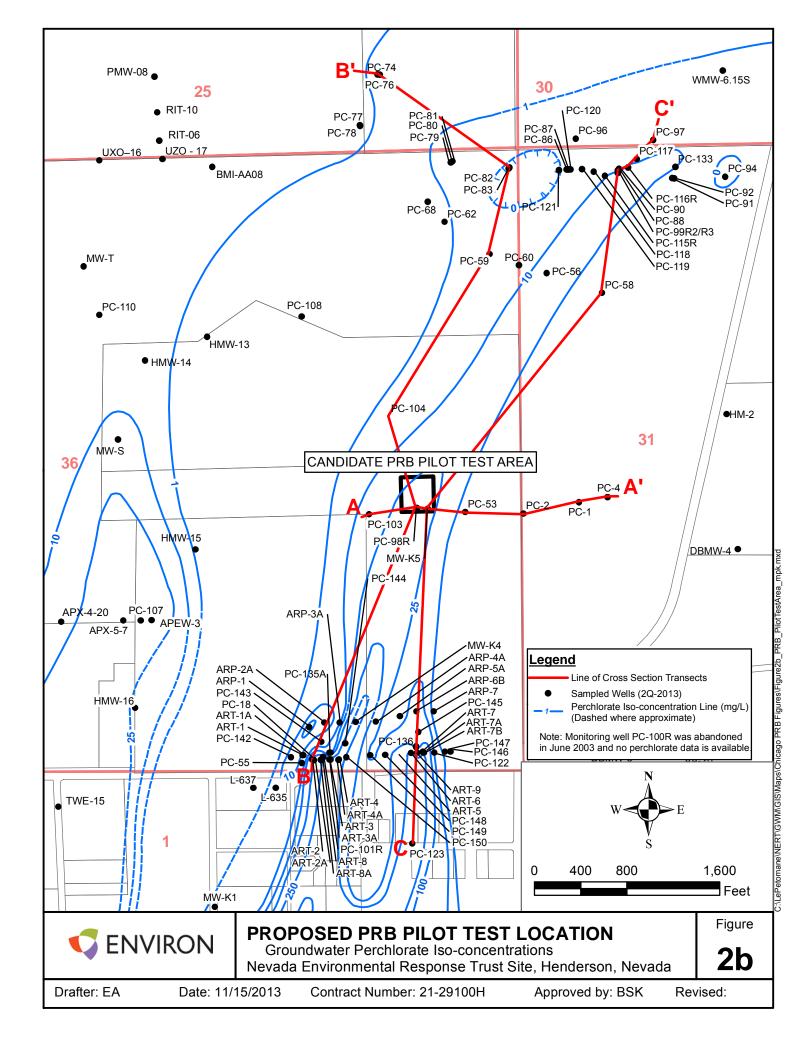
Method
Portable field instrument
Portable field instrument
USEPA Method 314
USEPA Method 300.1
USEPA Method 300.0
USEPA Method 200.8
USEPA Method 236.1/236.2
USEPA Method 415.1
USEPA Method 300.0
USEPA Method 300.0
HACH Method 8131 (USEPA Methylene
Blue Method
Method SW8015 Modified
USEPA Method 7199

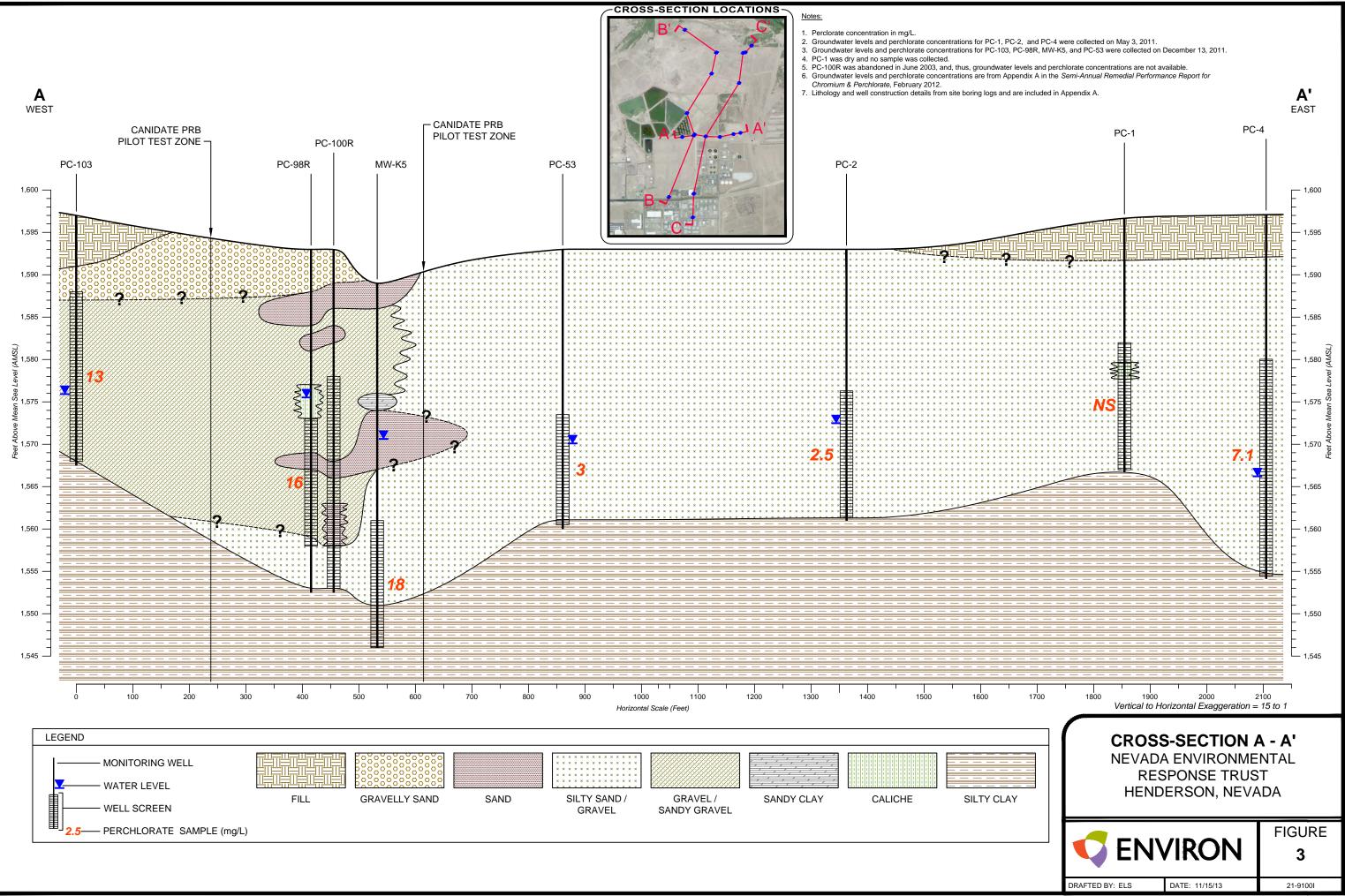


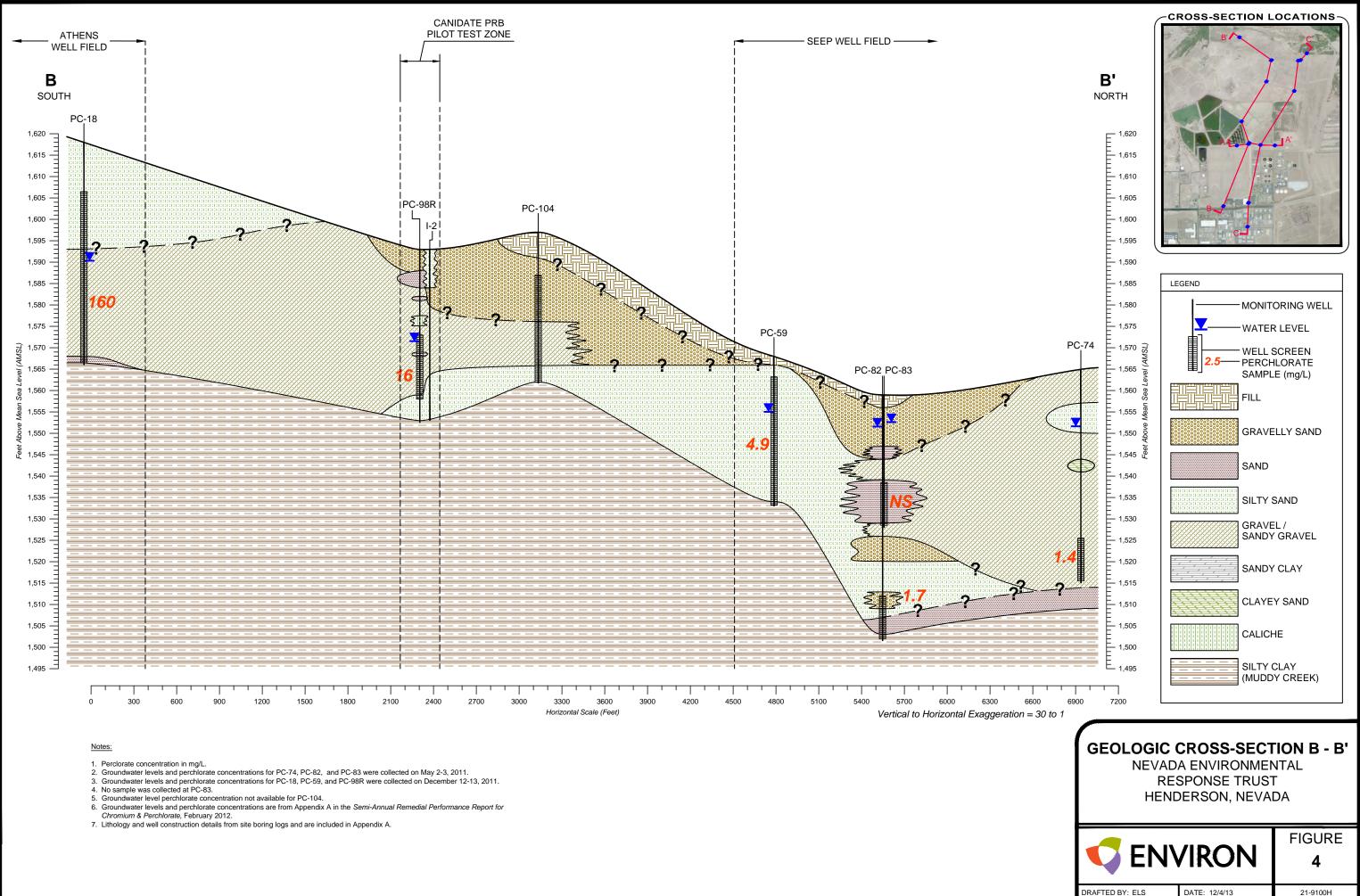
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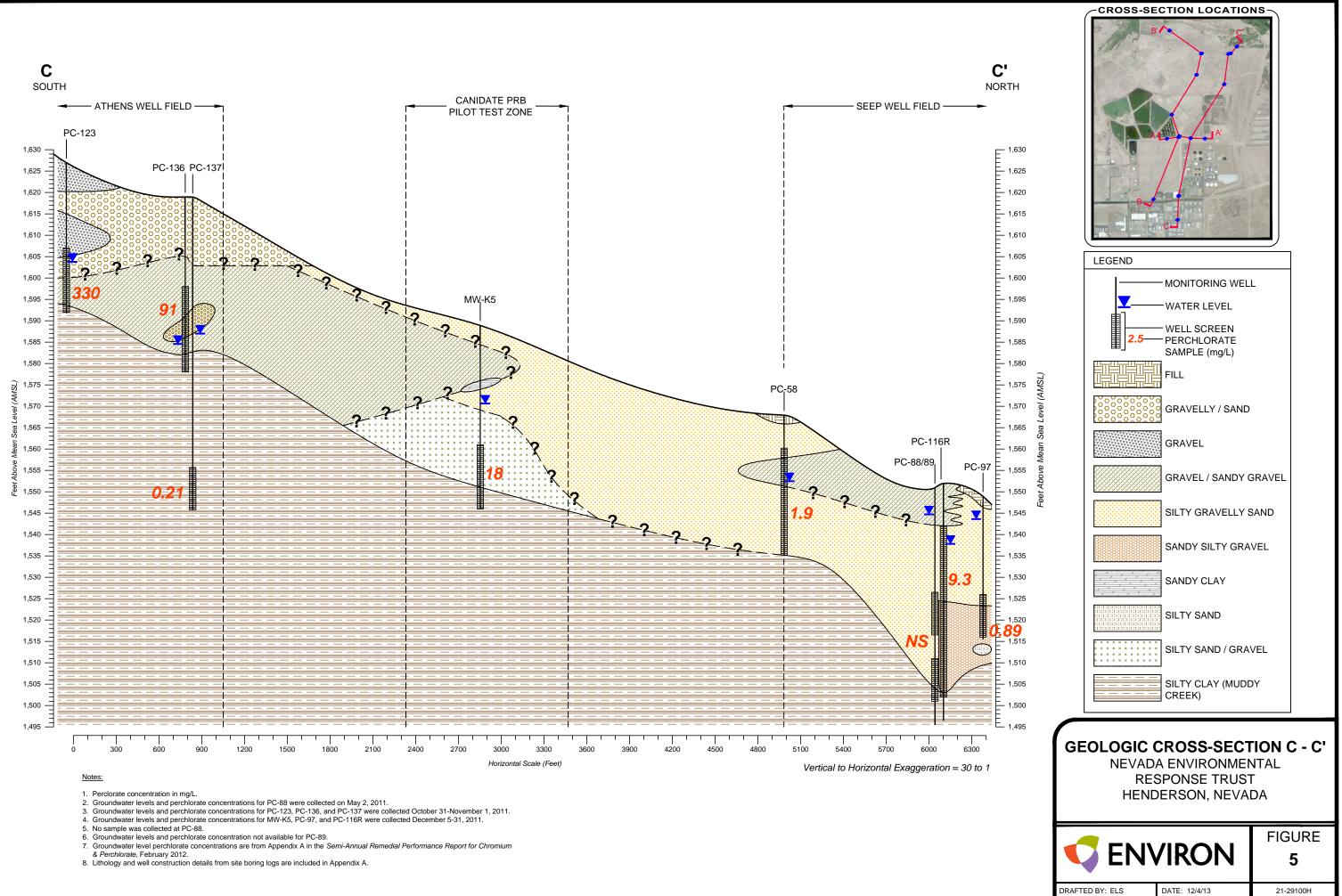












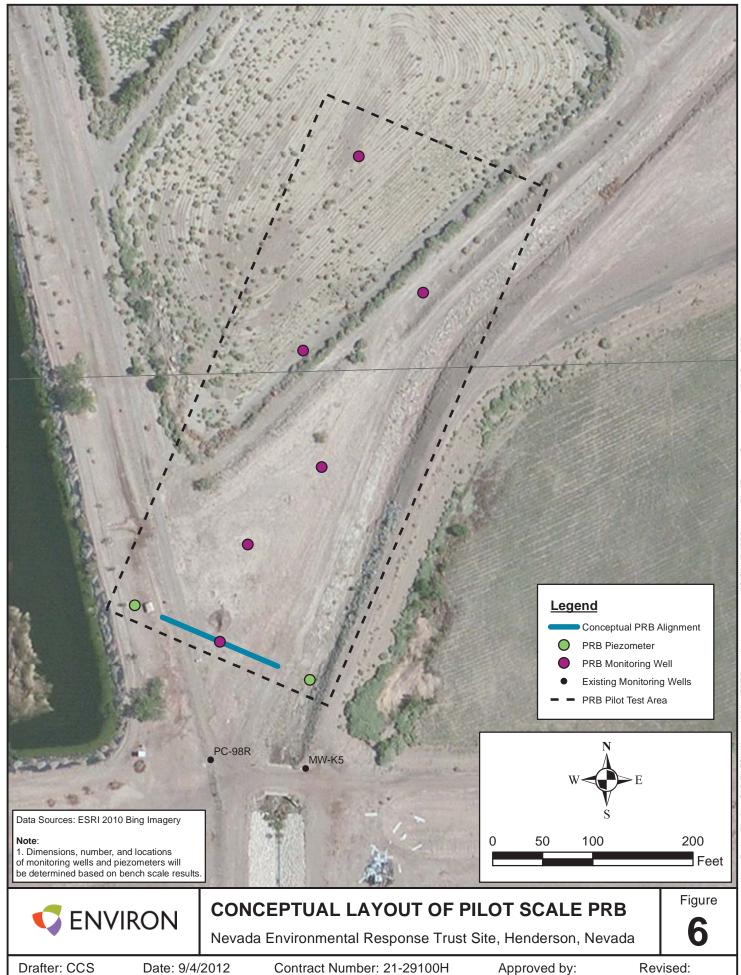
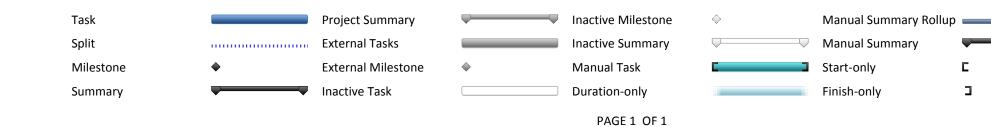
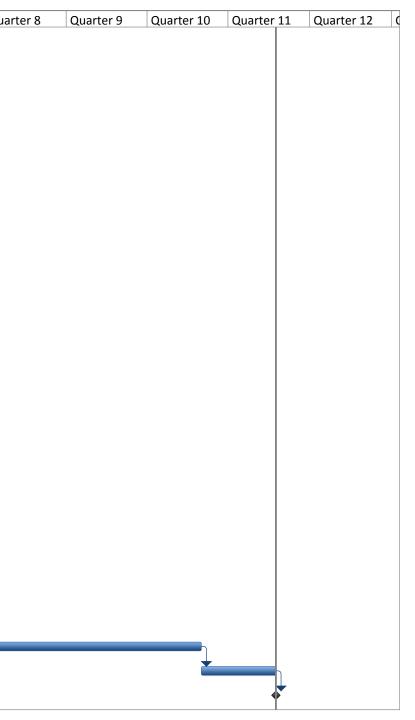


Figure 7. Preliminary Time Schedule for PRB Treatability Study

ID	Task Name	Duration	Quarter -1	Quarter 1	Quarter 2	Quarter 3	Quarter 4	Quarter 5	Quarter 6	Quarter 7	Quart
1	Work Plan Submittal to NDEP	0 days									
2	NDEP Review	60 days									
3	Respond to NDEP Comments/Finalize Work Plan	45 days									
4	NDEP Approval of Work Plan	0 days			* 1						
5	Coordination for Access with City of Henderson	60 days			(J				
6	Prepare and Submit UIC General Permit Application	2 wks			4]					
7	NDEP Review UIC General Permit Application	60 days	-		i	۲ ۲	J				
8	NDEP Issuance of UIC General Permit	0 days				•	•]	
9	Preliminary Field Activities	2 wks									
10	Single Well Dilution Testing	2 wks			ì						
11	In-Situ Microcosm (ISM) Studies	170 days				¥					
12	ISM Stage 1	50 days	-				₩				
13	ISM Stage 1 Study (minimum incubation of 4 weeks)	6 wks	-								
14	ISM Stage 1 Analysis and Reporting	4 wks	-				- 1				
15	ISM Stage 2	120 days					V				
16	ISM Stage 2 Study (estimated at 3 to 5 months)	5 mons	-								
17	ISM Stage 2 Analysis and Reporting	4 wks						— —	٦		
18	Bench-Scale Study	80 days									
19	Laboratory Column Testing (estimated at 8 to 12 weeks)	12 wks	-				•				
20	Data analysis and Reporting	4 wks	- •								
21	Finalize Field-Scale Pilot Design	60 days	•								
22	NDEP Review Final Field-Scale Pilot Design	30 days								* h	
23	NDEP Approve Final Field-Scale Pilot Design	0 days	- -								
24	Mobilization for Construction of Field-Scale Pilot	2 wks	- -								
25	Construction of Field-Scale Pilot	6 wks									
26	Field-Scale Pilot Operations	9 mons									±
27	Prepare Treatability Study Report of Field-Scale Pilot	60 days	-								
28	Submit Treatability Study Report to NDEP	0 days	-								







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Appendix A

Boring Logs and Well Construction Diagrams

EXPLORATION LOG MW-K5



OJECT: FORMER PEPCO			PROJECT NO.:	97664V1	
LE LOCATION: SEE SITE			EXPLORATION DAT	TE: 4-2-98	
EXPLORATION SIZE (diame G.S. ELEVATION:	ter):	2" MONHORING WELL 1592 49	LOGGED BY:	MOBILE B-61-HD	(
INITIAL DEPTH TO WATER: FINAL DEPTH TO WATER:	:	24 DATE MEA 18.7 DATE MEA	ASURED: ASURED:	<u>4-2-98</u> <u>4-3-98</u>	
LEVATION/ SOIL & SAMPLE UPTH SYMBOLS	uscs		DESCRIPTION		WELL CONSTRUCTION
		D. I. I. San and a second state			
1590-2.5	F	Dark brown poorly graded			
587.5-5	Г	dense.		a sana, moist anu	
	GP-GC	Dark brown poorly graded	gravel with clay an	d sand, moist and	
582.5 - 10		dense.			
1580 12.5	CL	Dark brown sandy lean cla	ay, moist and very s	stiff.	
1577.5 15	SP	Dark brown poorly graded			
+ ()		groundwater encountere	ed, medium dense t	0 22.0	

GEOTECHNICAL & ENVIRONMENTAL SERVICES, INC.

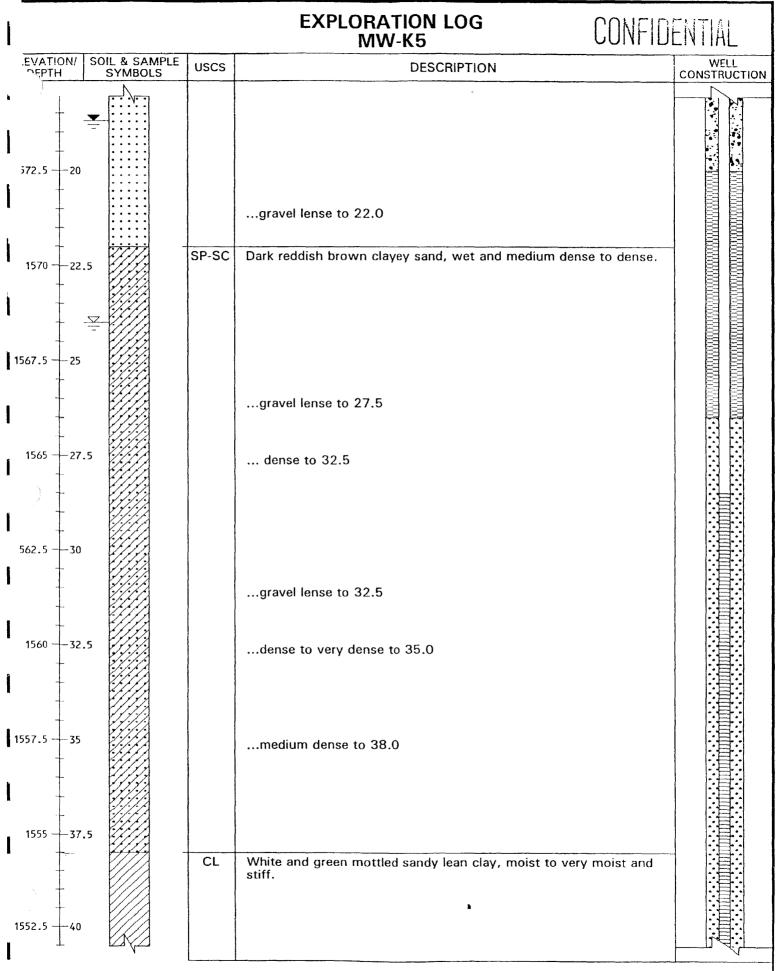
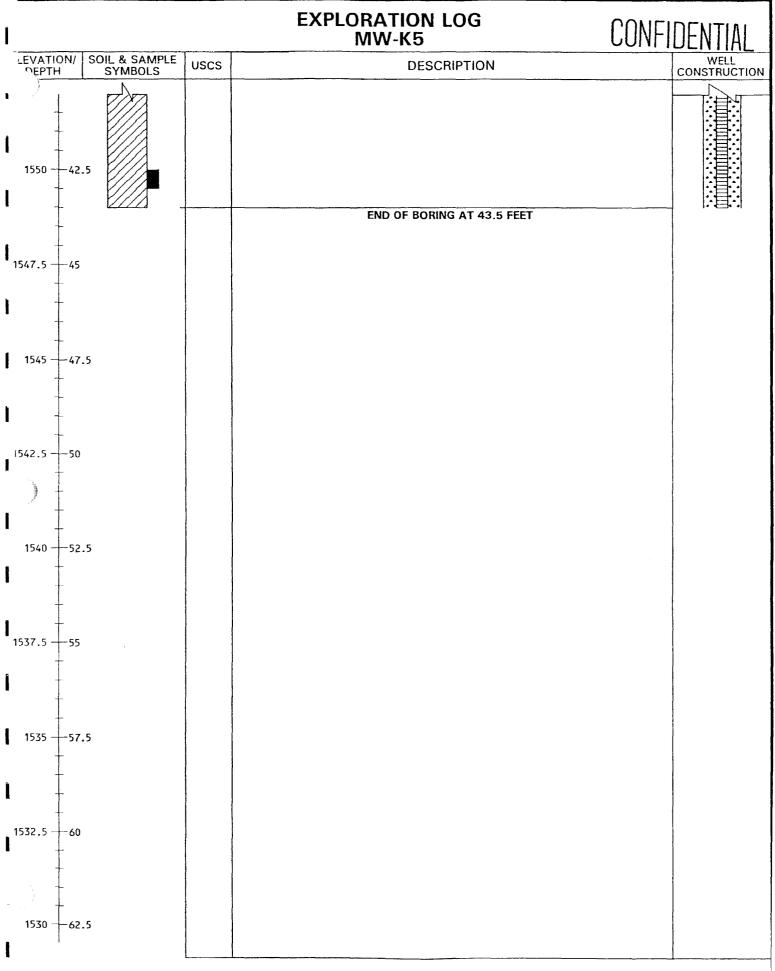


Figure No. 19



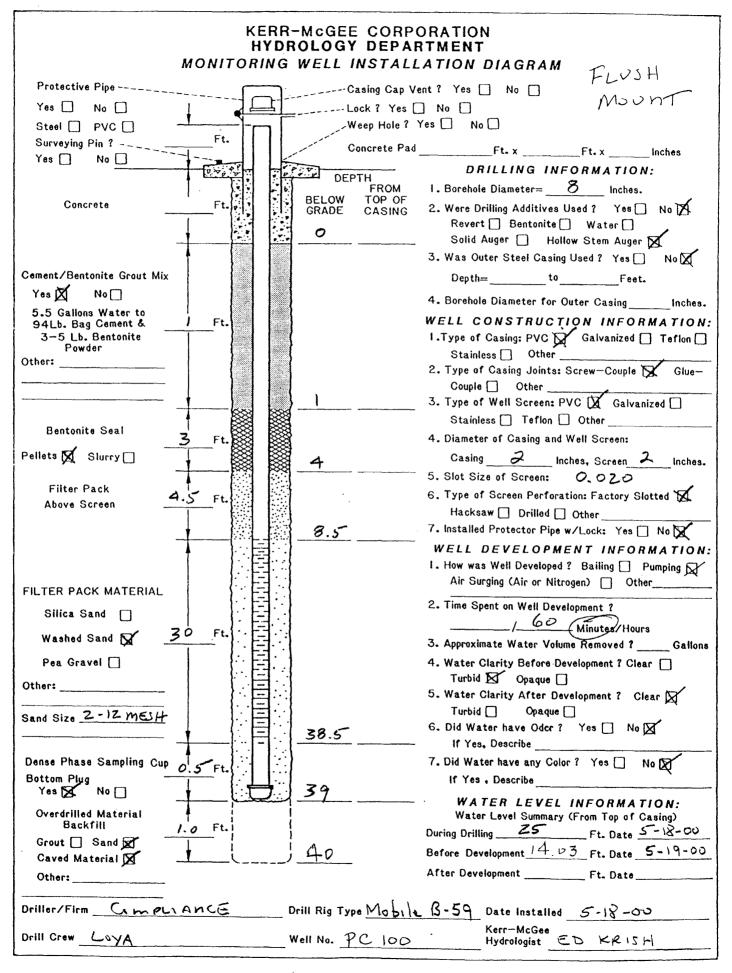
GEOTECHNICAL & ENVIRONMENTAL SERVICES, INC.

Figure No. 19

KERR-McGEE CORPORATION					HENDERSON, NV NUMBER					G		
Hy	drology Dept S&EA Division	KM			.		DER	50	NINV	NUMBER PC 100		
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IN FEET	LITHOLOGIC DESCRIPTIO	N	GRAPHIC LOG	SOIL FIELD CLASS.	PER 6'	(ppm)	NO.	ТҮРЕ	DEPTH	REC.	REMARKS OR FIELD OBSERVATIONS	
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	0-18 gravelly SAN		0									
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-	ZU-25% poorly sorted, xf-vc sd	5+1-54)	00	SW							_	
-	50% volc granules		-010 9									
25_	SUN VOIL grandle	s and	0								- V C 25' -	
	pebbles to 3"		010 0								<u>_</u>	
-	Locally hard thin		100									
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29-			000									
	29-36 SIty SAND, yell brn (10yR 6/4), v	, 1 + ,	•									
	w/com m- 29, SR-S											
	25-30% silt. Very			SM					-		-	
- 1	calcareous. Minor size caliche nodul	m-vc										
	size caliche nodul	ŧs										
36 ~			·. ·.									
	36-45 sity grant Si mod yell brn (10YR	AND,	0.10								_	
-	mod yell brn (103K	5/4)	0	SW							-	
	25% silt, 25% volcg	ranult	0.0									
Y	Water Table (24 Hour)				G	RAPHIC L	OG LEC	GEN		ORILLED	PAGE	
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	WALLED CONTINUOUS TUBE SAMPLER		COVER	Y		CLAY	\Box		EXIST	ING GRAD	ELEVATION (FT AMSL)	
DE	PTH Depth Top and Bottom of Sa	mple			RTT I	CLAYEY SILT	Π.		LOCA	TION OR C	GRID COORDINATES	
R	EC. Actual Length of Recovered S		Feet			UTC 1						

KERR-McGEE CORPORATION Hydrology Dept S&EA Division				LLC	-	HEND	ERS	50 r	J.NV		G ER PC 100
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f FEE		N	GRAPHIC LOG	SOIL FIELD CLASS.	PER 6"	PID (ppm)	NO.	ТүрЕ	DEPTH	····	REMARKS OR FIELD OBSERVATIONS
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	(5YR 6/2) 10% clay	, 20%	1								MC not -
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EXPLANATION			ORE			GRAVEL	[]	CLAN SAN	rey D	ED 1	<rish< td=""></rish<>
μ Ω	THIN- WALLED TUBE) Cover	Y	53	SILTY			Ē	XISTING GRAD	E ELEVATION (FT AMSL)
	DEPTH Depth Top and Bottom of Sa REC. Actual Length of Recovered S	mple Sample in	Feet			CLAYEY SILT				DCATION OR (GRID COORDINATES

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	RR-McGEE CORPORATION KM S drology Dept S&EA Division	KMC	LL	C	Henc	lerse	<u>n</u>	,NV		BORIN	PC 100 R
DEPTH		10	UNIFIED					DIL SA			
IN FEET	LITHOLOGIC DESCRIPTION	GRAPHI LOG	SOIL FIELD CLASS.	PER 6'	PID (ppm)	NO.	TYPE	DEPT		REC.	REMARKS OR FIELD OBSERVATIONS
_	0-4 gravely SANI	>, 0:0:									start drilling _
	gry brn w/ 10-15% sil	+ 0.0	SW		 						@ 8:30 am _ finish @ 9:00 _
	ZO-30% volc granule pea gravel. Vf-vc SF	5 to 0.0.									+inish e 9.00 _
4 -	pea gravel. Vf-vc SA	sd::::									
<u></u>	4.7 SAND, gry brn.	/	SW		<u>├</u>						. —
7 -	10% silt and 5-10% v.	sm 0.00			<u> </u>						
9 -	granules to Vio". f-V	<u>د انمن</u>	GW								
	SA-SR sand.		SW		<u> </u>						
// 1	7-9 Sdy GRAVEL, b A-SA to 1". 30-35%	1080									dampeiz!
-	1 - 50 - 55 %				L						
-		1/2 0000							•		
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	F-ra, SA sand	0.0.0	GW								
-	11-25 Sdy GRAVEL	-00	000								<u>Ve18'</u>
	brn w/ 5-10% silt +	0.0.0			<u>}</u>						
20-	25-30% vf-vc, SR-SAS										
	Grav. up to 2" (ave Vio		1		<u> </u>						
-	3/4") volc w/minor		•								-
	caliche coatings	0.0.0									
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-	w/ 10-15 L sm vole gran	vles 000	GW		E						-
30-	INF-VC, SA-SR					ļ					
-	27-30 say GRAVEL	1.1			-						
-	bm, volc up to z" (ave ! clean, vf-vc sd	14	SW								-
_	30-35 SAND, brn, VI	-c									-
35-	w/minorve, SA-SR. 10-15	Z= 1.									
	35-29 ELL SAND/SAVE		SM	l							-
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	Isa-sasd	1-10 1-10	GM/			}					-
J	Water Table (24 Hour)		<u> </u>	G	RAPHIC	OG LE	GEI	ND		RILLED	PAGE
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ZPLA					GRAVEL		SAN	D			KRISH
(^{iui})	THIN- WALLED CONTINUOUS TUBE SAMPLER	RECOVE		SILTY CLAY			EAISTI	ISTING GRADE ELEVATION (FT. AMSL)			
	EPTH Depth Top and Bottom of Sampl			87	CLAYEY SILT				LOCAT	TION OF	
	REC. Actual Length of Recovered Sam								<u> </u>		

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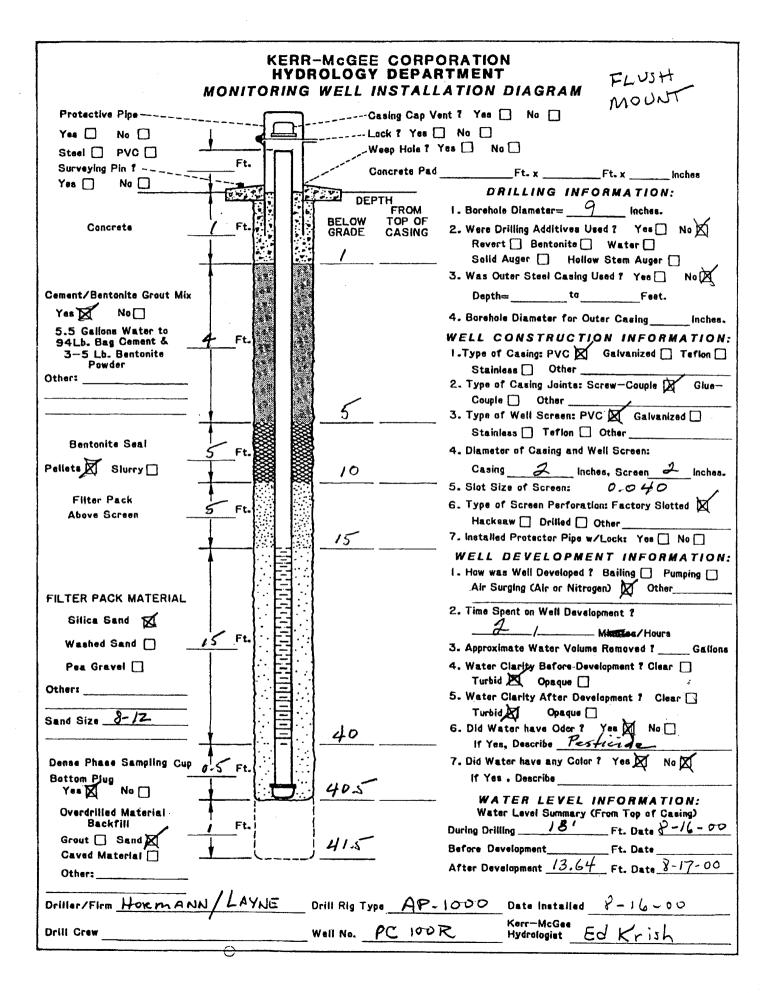
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	RR-MCGEE CORPORATION Irology Dept S&EA Division	KM SUBSIDIARY	LLC	H	ENDER				NG PC100R
DEPTH IN FEET		Z GRAPHIC	UNIFIED SOIL FIELD CLASS.	BLOWS PER (P 6" (P	PID pm) NO		OIL SAN	······	REMARKS OR FIELD OBSERVATIONS
4.5 <u>1</u> 	38-40.5 gravelly SAND, brn. 20-2 SILL and 10-2070 V Sm granules. SA-SA Ve sd. <u>40.5-41.5</u> H grn CLAY W/ gyp xta TD 41.51	$\frac{1}{2} \frac{1}{2} \frac{1}$							- MC @ 40.5 '
	Water Table (24 Have)			GRAP	HIC LOG	FGFI			
)				DEB		8-16	-00 2 of 2
PID NO	 Photoionization Detection (pr Identifies Sample by Number 	om)					Y		LUSSION
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EXPLA			•	GRA		CLA	ID	ED	KRISH NDE ELEVATION (FT. AMSL)
	WALLED CONTINUOUS TUBE SAMPLER		DVERY]			
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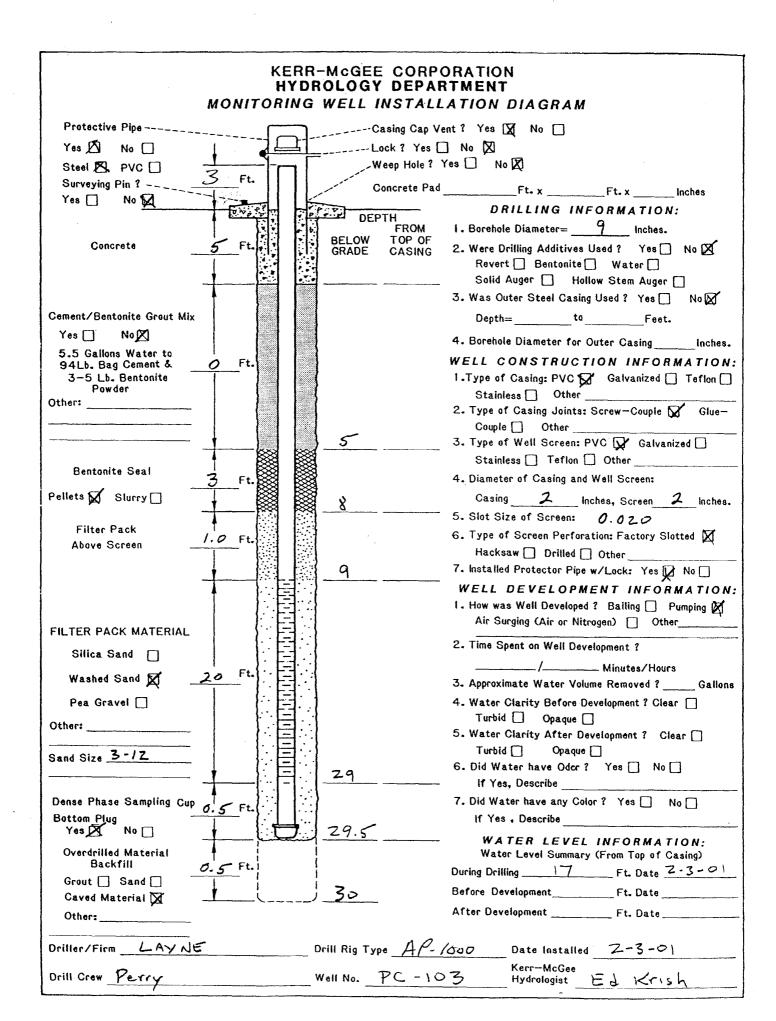
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	RR-McGEE CORPORATION drology Dept S&EA Division	L	LC		Hend	lers	on	, M		er PC 103	
DEPTH IN FEET	LITHOLOGIC DESCRIPTIC	м	GRAPHIC LOG	UNIFIED SOIL FIELD	BLOWS PER 6'	PID (ppm)	NO.	μ.	DIL SAMI DEPTH	PLE REC.	REMARKS OR FIELD OBSERVATIONS
	<u>0-6</u> BERM. Com construction mal		5	CLASS.				E			
- 0 - - 10	6-10 SAND, grav. brn (5485/4), 10-20% vol grav to 1/4 " in rf-rc, A	elly, é pea srsd	0.0	รม							
	10-17' GRAVEL, SO sity, brn, 10-20% site 20-30% vf-ve, SA-SR & Vole gravel to 14. SI careous. [Prob. series fining-up. alluvial beds 16-17' Gravel to 4"	t is d in . cal-	0.000,00,00,000,000 0.000,00,00,00,00 0.000,00,00,00,00,00	GM							damp @ 14' -
-	17-29 Gravel, 51. +r silt. 10-15 % vf sd, A-SR, volc 5A-S gravel to 1/2" w/ loca beds up to 4" 25-28' com 1g volc to 4"	-VC R pea 1 thin	000	GP							WTR E 17'
	<u>Z8-29'</u> gravel w/ siltin matrix <u>Z9-30'</u> CLAY, si È CLAY, l+ grngr (56Y 8/1), 10-20 [s in matrix, non-c	20-302 11+4 144 144 144 144 144	0000	CL							MC & Zq /
	EUUS, HT-SPOJPS TD 30' Water Table (24 Hour)	- u m			G		OG LE	GEN	U		
	 Water Table (Time of Boring Photoionization Detection (pp Identifies Sample by Number 		COVER	Ŷ		CLAY SILT SAND GRAVEL		DEBR FILL MGHLY DRGAN SANI CLAY	RIS DF NC (PEAT) DF DY ((C D) EX	LAV LAV Ed	NE

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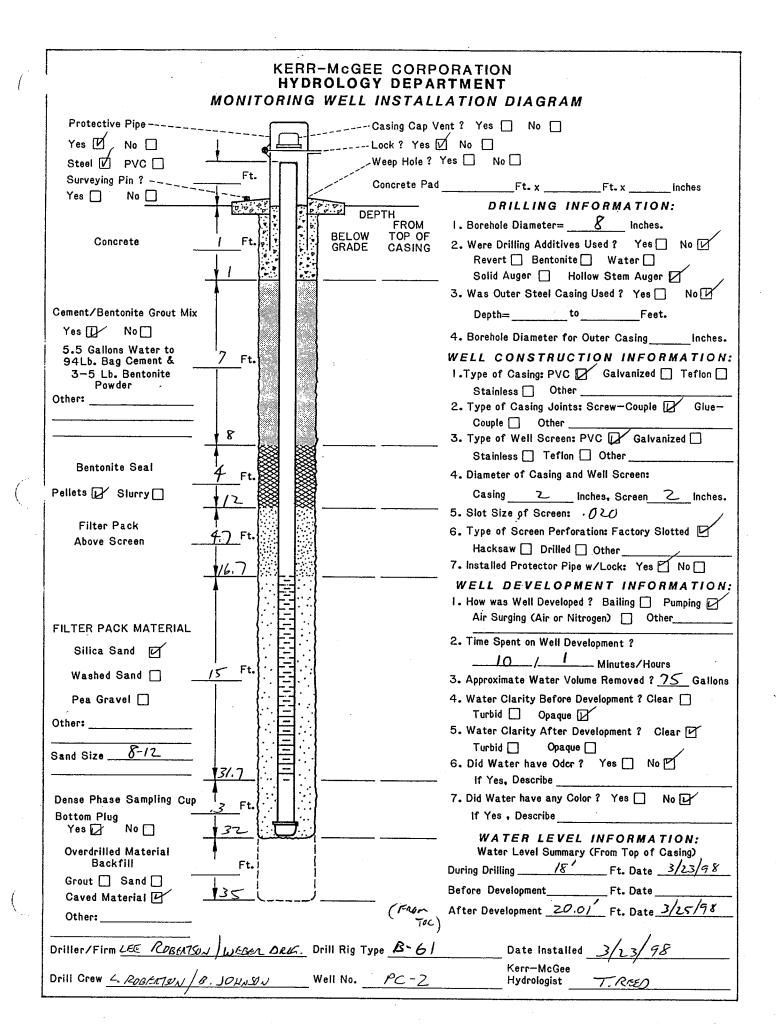


	KERR-MCGEE CORPORATION						LOCATION				BORING		
	Hye	drology Dept S&EA Division	KM	<u>cuc</u>		T	HUSE	1.0~	\sim	V	NUMBER PC-2		
1 1	PTH N	LITHOLOGIC DESCRIPTIC	N	GRAPHIC LOG	UNIFIED SOIL FIELD	PER	PID (ppm)			IL SAMP	· · · · · · · · · · · · · · · · · · ·	REMARKS OR FIELD OBSERVATIONS	
FE	ET			ອີ	CLASS.	6'	(PP)	NO.	TYPE	DEPTH	REC.		
	_	SAND/SILTY SAND W/	ABD	1 2					ũ			-	
Í	-	GRAVEL ; LT. TAN- BR		0º		5							
	-	WALL - GRADED; DRY	-										
15				U									
	_	GAMVEL C 6-7'		O O					R.		1.		
	_			800									
	_				sm-							-	
10		SAND AS ABOVE		0	GM	ļ							
10		JANG AS MOOVE			G/ ·								
		,		\$ 0								-	
		GRAVEL C 14-15										-	
1,-	. –			800								•	
15													
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1	8				I.							-	
120		SAND AS ABOVE, SATURA	てい	1 do					語			-	
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	_			Íd									
25				00								-	
(-)				cT.					ALC: N				
	_											SAMPLE TAKEN -	
	-			U	1		<u> </u>		ALC: U		ł	C 30' -	
30				ot					2				
3		····		1.0		3 ₁₂ 27		1	Х	30 31,5	1,4'	_	
		SILTY CLAY, REDOUGH - ON		NAV	1								
		GRADING INTO LT. GRA MUDDT CREEK	1- Call		CL							-	
35		MODEL LIVER		1/1	1					*****			
F		TO 35'										_	
		12 23		1								-	
							<u> </u>						
		Water Table (24 Hour)					RAPHIC L			<u> </u>	TE DRILLED	PAGE / of /	
))				CLAY		DEBRI FILL	S	LLING METH		
	NC TYP	 Identifies Sample by Number 	r				SH T	Ţ,	IIGHLY)RGANI(145,	A	
101						1				· · · · ·	ILLED BY		
EXPLANATION	X	SPLIT- BARREL AUGER	RC	DCK DRE		1	SAND			LO	GGED BY	r Drichlah	
(PLA							GRAVEL		SAND		T, R	<i>VED</i>	
Ê		THIN- WALLED CONTINUOUS TUBE SAMPLER		O COVER	۲Y	\boxtimes	SILTY CLAY			EXI	STING GRAD	E ELEVATION (FT. AMSL)	
	DE	PTH Depth Top and Bottom of Sa	لا mple			1	CLAYEY SILT			LO		GRID COORDINATES	
	RE	C. Actual Length of Recovered	Sample in	Feet						_			

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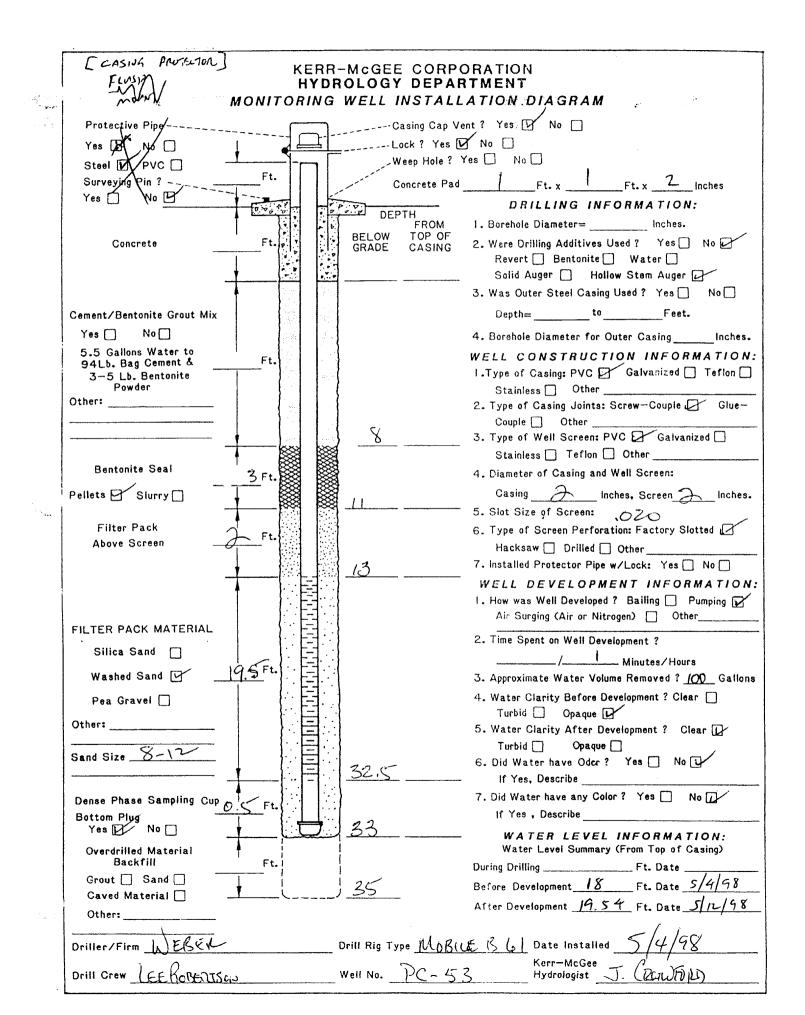


SOIL	BOR	ING	LOG	KM-5655-

		KERR-McGEE CORPORATION Hydrology Dept S&EA Division	KM SUBSID		(LOCATION	モロくへ	, d.	B N	ORING UMBER	PC-52	2
- 13m			inc		UNIFIED	BLOWS		T	SOIL SA		.7		
	IN FEI	I LITHOLOGIC DESCRIPTIC	N	GRAPHIC 10G	SOIL FIELD CLASS.	PER 6'	PID (ppm)	·			EC.	REMARKS O FIELD OBSERVAT	NIONS
		- SILTY SOUD WI GR	AUFL	1-0-			-						
		- LT TAN - TREN ISTEN		1	51.1								
		WEY GRADED DRY		0	SM/								
	5	-		10	Gan								-
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	10			:10									-
		-		0-1-									
				6									
	15	, 		ن • • •									-
				Ve									
		- JAND/GROVEL DOD	r RDA	¢ -								_	
	22	- JAND/GROJEL DATE CLOYEY MOIST BOU	<u> </u>	υ.							4 N	ATER	
		- WEt	- (SAUDLE	
~		- 5000 (DS-11625 DI	L KRU	1	<i></i>							GLELTED	
	2	- FAND (DS-UCRS DI TR GREVEL			Sh							At 20'	
	122	-		_! ·									
		_											
	30												-
	0	,		1							۲ <u>۲</u>	33 T/ MUDD	7
	33	- sicty cury is any is		202	CL		<u></u>					(REE	<u>íc</u>
		- white have soft to F	17cm									D 341	
		_										D Set	
•	F	V Water Table (24 Hour)		1	l	GI	RAPHIC L	 OG LEG	END	DATE DRI		PAGE	
		V Water Table (Time of Boring						DI EXE FI			4/S	8 / of	1
		PID Photoionization Detection (pp NO. Identifies Sample by Numbe TYPE Sample Collection Method							GANIC (PEAT)	HS	2		
	ATIO			0.5%				S c			BY EDEL	~	
	EXPLANATION	SPLIT- BARREL	RC	OCK ORE		1	GRAVEL			LOGGED	~	(150)	
	EXF	THIN- WALLED CONTINUOUS THEE SAMPLER		O ECOVER	Y					-		LEVATION (FT. AMSL)	
		DEPTH Depth Top and Bottom of Sc	Ľ			1	CLAYEY SILT			LOCATIO		COORDINATES	
		REC. Actual Length of Recovered		Feet		נעננט ו	SIL I	L	····				

1	KERR-McGEE CORPORATION KM SUBSIDIARY Hydrology Dept S&EA Division KMC-LLC					HENTRESON NV					BORING PC-53		
DEPTH IN FEET		N	GRAPHIC 10G	UNIFIED SOIL FIELD CLASS.	BLOWS PER 6"	PID (ppm)	NO.	шТ	SAMPLE DEPTH	REC.	REMARKS OR FIELD OBSERVATIONS		
10 5 7 7 7 7 7 7 7 7	SILTY SAND RO BR TO TAN GRAVELS WELL GRADED DRY SILTY SAND WI GRAD BCOLG CLAYEY MOIS DARK BRWD	EL											
30	SAWD SPLEY BRN-I BURN SLI CLOYEY TR GULDVELS G2S-UC SOT SILTY GOY GRN GJ TO LAW FIRM	ils Giz		X							B2'T/MUTDY CREGIL		
EXPLANATION	Water Table (Time of Boring D Photoionization Detection (p D. Identifies Sample by Numbe		DCK DRE D COVER	Y		SILT SAND GRAVEL		SEND DEBRIS ILL IGHLY IGALY IGALY IGALY IGALY ICLAY ICLAY ICLAYEY					

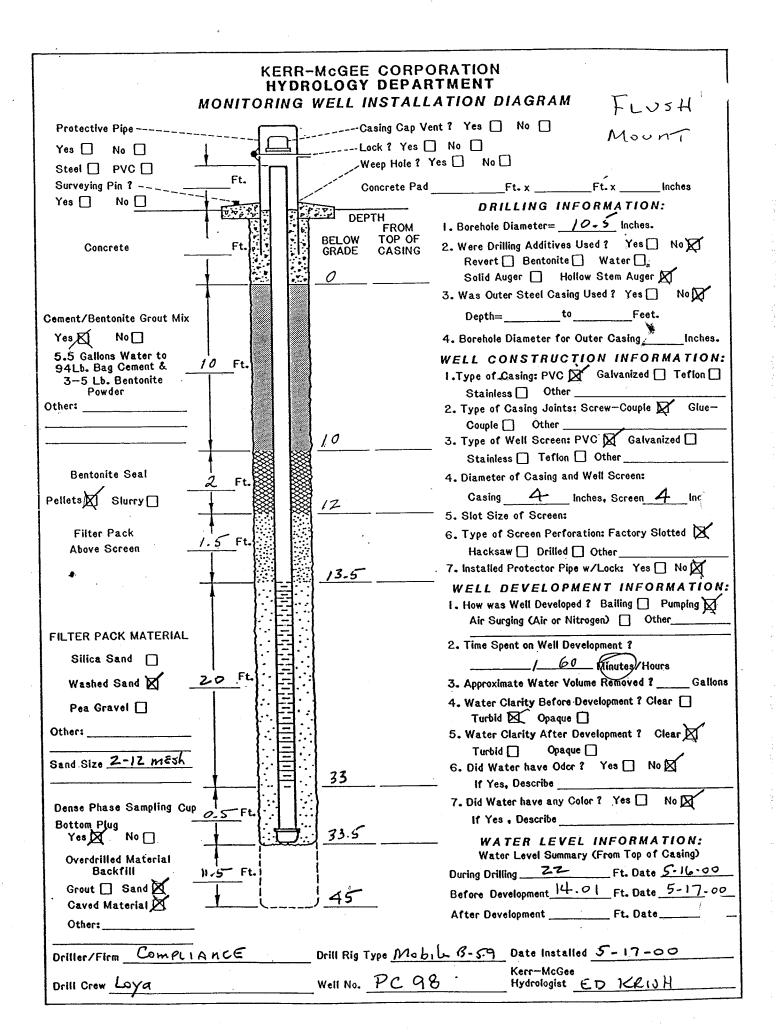
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	RR-MCGEE CORPORATION	KM SUBSIDI		LC		HENDI	E la < a	.1	NV		RING	; R PC	98	
<u> </u>	drology Dept S&EA Division	INME			NOUS		LRSO					10	10	
DEPTH IN FEET	LITHOLOGIC DESCRIPTIC	и	GRÀPHIC LOG	SOIL FIELD CLASS.	PER 6"	PID (ppm)	NO.	TYPE	DIL SAMI DEPTH	RE	c.		AARKS OR BSERVATIO	45
_	0-12 gravelly SA	NR.	6 0 -											_
_	mud yell brn (10YR					 								
-	20.757 annules é	cm.				<u> </u>								
5_	20-25% granules à pebbles to l'diam	(volc)		. –			ļ							
–			00	SP		<u> </u>								4
-	Sp-mod silt in mat		0.0.			<u>. </u>				-				
	zoz.). Sand vf-v	C ,SR-5A	0											_
10 <u> </u>			:0			\vdash								
												+		
-	12-16 sity soly GRAN	VEL	00.0			\vdash								4
-	Hbrn (5YR 5/4) 20	Zsilt	000	GW		-	1				``	dam	PEIS	-
16 -	25% VE-VL A-SR	sand.	000		ļ				ļ				•,	
1	50 % volc granules -	ю	<u>[0]</u> -			<u> </u>								
	"I copplex up to 6"	diam				 -								-
20_	Mod com caliche +	tronghi	100											
-	13-16 v. hard, dens.		00	1		-							ezz'	-
-	calichi fication			1								·¥		· _]
	16-34 sity grav	elly	0.0											_
25-	SAND, mod brn (54 RA/A	계약	50		\vdash								
-	20-25 % 511+, 20-25	5%				Ľ								
	granules and sm pe					 								
_ ·	103/4". 50% vf-vc sand	A-SR	.0.0.											
30-	, iana													
						-								-
- 1			0			\vdash					3			_
34	34-37 SILY SAND	14 yel						Τ						·
	- brn(104R 6/4). 4f	- fg w/		; sm		-								
37	- minor mg, SR-SA. - silt. Mol com ma	25-302	1নি	·	+	-		╈	-					
	- nodules. Very cale	Acesus		GC	·+									<u>ــــــــــــــــــــــــــــــــــــ</u>
			1.0	IML	<u>. </u>	GRAPHIC	LOG L	EGI		DATE DR	ALLEO	<u> </u>	PAGE	
1 1	✓ Water Table (24 Hour) ✓ Water Table (Time of Boring)	naì			-	CLAY			BRIS	5-		- 07) HOD	1 of Z	<u>'-</u>
1	PID Photoionization Detection (VQ. Identifies Sample by Numb	ppm)				SILT			HLY				HJA	
	YPE Sample Collection Method					-	<u>ک</u> ا ا		GANIC (PEAT)	DRILLED	BY	<u> </u>		
EXPLANATION					1	SAND				LOGGED	D BY	Lomf	MIANCE	
(PLA						GRAVEL			AYEY ND			KRIS	-	<u></u>
ŵ	WALLED CONTINUOU TUBE CONTINUOU	JS	NO RECOVE	RY		SILTY CLAY]_		EXISTING	G GRA	DE ELEVAT	ION (FT AMSL)	
	DEPTH Depth Top and Bottom of	لاا Sample			R	CLAYEY SILT]		LOCATK	ON OR	GRID COOL	ROINATES	
	REC. Actual Length of Recovere	d Sample	in Feet											

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	ERR-McGEE CORPORATION drology Dept S&EA Division	KM SUBSIDIA		.LC		LOCATION HENDE	- Se ta	2		BORING	G R PC	98
DEPTH	I		υ		BLOWS				DIL SAME			
IN FEET	LITHOLOGIC DESCRIPTIC	м	GRAPHI LOG	SOIL FIELD CLASS.	PER 6'	PID (ppm)	NO.	TYPE	DEPTH			ARKS OR SSERVATIONS
41 -	37- 41 sdy grav SILT	-/s1+y	1010					_				
-	grav SAND W/ 15%	dissem	WA								MCO	@ 41′_
-	grav SAND w/ 15 % 's granules to 1/8-1/4"	mod	\mathcal{M}	CL		 						
15-	gry orange pink (5y	R 6/2)	<u>///</u>									
- 21	Contains 25-50 % vi					L						_
-	In silt/clay matrix.1	0-20%								- ,		
	Volc +1s granules to 1/8	-14".										
	Very calcareous w/m	od c-ve				 						
- 1	caliche nodules.											
-	41-45 SHy CLA	۲,										_
	1+ gmgry (5648/1) m											
_	gry (568/1). 25% sil					<u> </u>						<u> </u>
-	sized caliche nudules										-	
	- siger criter portes i		ł			<u> </u>						-
	45'17					—]	-
												
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				<u> </u>							<u> </u>	PAGE
11	Water Table (24 Hour)					GRAPHIC			and	5-16 -	1	Z of Z
	Water Table (Time of Borin PID Photoionization Detection (p	շթտ)				CLAY				DRILLING MET		
	10. Identifies Sample by Numb YPE Sample Collection Method					SILT		higi Org	AAHC (DEAT)	DRILLED BY	HSA	
		n .	0.5%			SAND	\boxtimes	SA CL			PLIAY	16
EXPLANATION	SPLIT- BARREL AUGER		OCK ORE			GRAVEL				LOGGED BY		
EXPI		s Ñr	10		4		لاۓ ()	SA		EXISTING GRA	KR 15	
		R	ECOVE	RY					——			
	DEPTH Depth Top and Bottom of S REC. Actual Length of Recovered	Sample I Sample in	n Feet			CLAYEY SILT				LOCATION OF	GRID COORD	



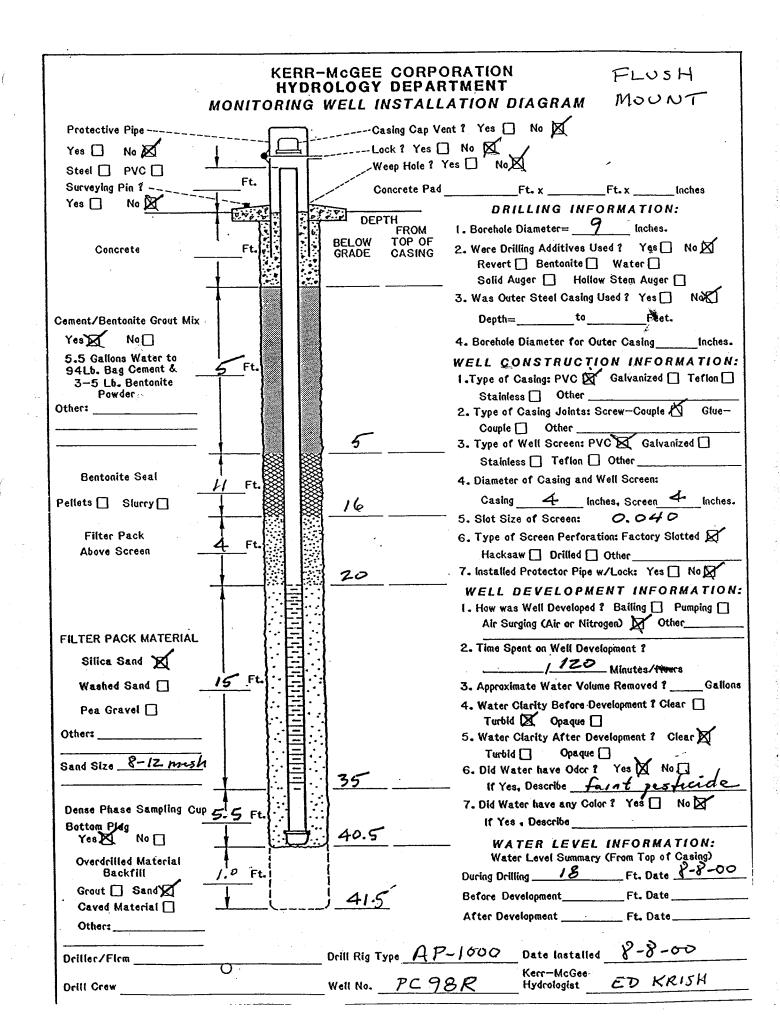
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	RR-McGEE CORPORATION drology Dept S&EA Division		<u>100</u>			<u> </u>	der	` \$0	in NV	BORIN	GR PC 98R
DEPTH IN FEET	LITHOLOGIC DESCRIPTIC	м	GRAPHIC LOG	UNIFIED SOIL FIELD CLASS.	BLOWS PER 6'	PID (ppm)	NO.	TYPE TYPE	DIL SAMPLI DEPTH	REC.	REMARKS OR FIELD OBSERVATIONS
-	<u>o-5</u> gravelly 5 gryish brn w/10% Si zo-30% granules- gravel to 5/4". vf-vi	i 1+ <u> </u>	0.00	SP							
5	5-9 SAND, guy & w/ 10% silt and 5-1	o7o		รฟ		 					
ן אין אין	volc granules to 1/4" SA sand 9-10 Sdy GRAVEL		00.0.	ฐม รม							
1Z -	ZJ-JS& vf-vc Sd 10-12 SAND, brn 10% silt, 5% granules volc, SA sand		100000 00000 00000	~~~						``	damp@12' -
-	12-24 sdy GRAUE brn. w/5-10% silt VF-VC, SA-A sand	, 25%	00101000000000000000000000000000000000	GW						_	7@18'
Z0	Granules to peagre A-SA. Yo"-3/4" w/ 1 3/4"-Z" Locally caliche · cemented.				6305			X	zo- z1.5	50%	
24-	cemented. <u>16-20'</u> hard. Com co cement			SP	25 50			X	25-26	75%	
-	<u>Z4-Z6</u> SAND. gry by <u>clean, f-mg w/c-</u> <u>Z6-34</u> sdy GRA gry brn, 10-15 20 sil 30% vf-vc, SA sand granule - pea grave <u>y</u> ² -3/4	<u>vcg</u> VEL 1,25- in		GW	22930			X	30'- 31.5'	B0%	
	29-30 - cobbles u 34-40.5 gravelly sl 20-30 % silt and 10 vole granules to 1/4" com. dissem st-size	+4 5AND -15-10		GMY SM	12 13 31			X	35'- 36.5'	100%	
	Water Table (24 Hour) Water Table (Time of Boring D Photoionization Detection (p O. Identifies Sample by Numbe	g) g)		<u>-</u> 1		CLAY SILT SAND		DEB FILL HIGHL ORGA			To 1 of Z
	SPLIT- BARREL AUGER THIN- WALLED TUBE EPTH Depth Top and Bottom of S REC. Actual Length of Re@vered	ample	OCK ORE IO ECOVE	RY		SAND GRAVEL SILTY CLAY SILT	হিহ	CLA CLA SAN		STING GRA	O KRISH DE ELEVATION (FT AMSLI GRID COORDINATES

soil bor	ING LOG	KM-5655-B
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	ERR-McGEE CORPORATION ydrology Dept S&EA Division	KMSUBSIDI		<u>, , , , , , , , , , , , , , , , , , , </u>		LOCATION	DERS	50	N NV	BORIN	G R PC	96	BR
DEPTH		<u> </u>	U H H	UNIFIED		PID		sc	DIL SAMPI				
IN FEET	LITHOLOGIC DESCRIPTION		GRAPHIC LOG	SOIL FIELD CLASS.	PBR 6"	(ppm)	NO.	TYPE	DEPTH	REC.	FIELD	MARK: OBSERV	ATIONS
40.5	-1		XX	ÇL								·····	
ļ .	Calcareous, Sand												-
	Vf-fw/minor mg, S												-
	40.5-41.5 sty ch	- 1				'							-
	It grn, w/ dissen	n sm											
	gypour xtals								-				-
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	TD 41.5												-
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1 1	Y Water Table (24 Hour) ✓ Water Table (Time of Boring)	a)				CLAY				1.8 - 8	-00	Ζ	of Z
	PID Photoionization Detection (pj NO. Identifies Sample by Numbe	pm)			1	SILT			4Y	Per	-0221	oN	
NOL	YPE Sample Collection Method	13 1			-	SAND				RILLED BY	AVN		
EXPLANATION	SPLIT- BARREL AUGER		OCK ORE		1	GRAVEL				OGGED BY	> KR		
EXP	THIN- WALLED CONTINUOUS	, Na	io Ecove	RY	1	SILTY		JA		KISTING GRA			MSLI
	TUBE SAMPLER DEPTH Depth Top and Bottom of Sk REC. Actual Length of Reförered	لالے ample				CLAYEY		<u> </u>		OCATION OR	GRID COO	ROINATES	

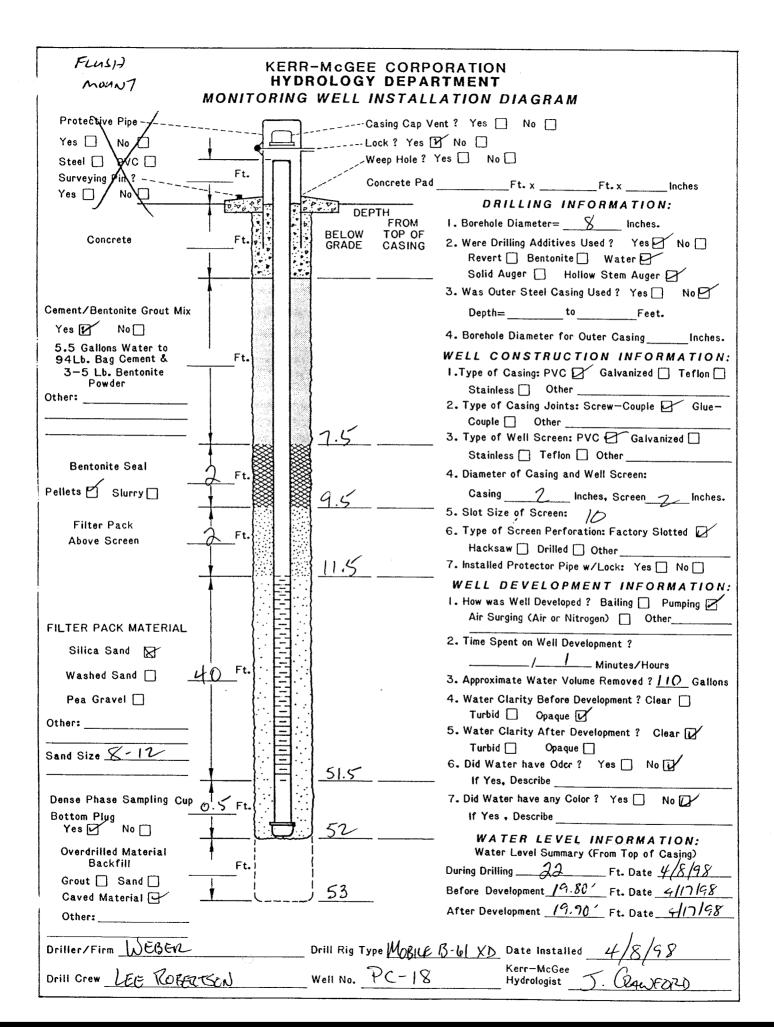


G	nor	thga ental mana					Boring Log	Northgate Environmental Managerr 24411 Ridge Route D Laguna Hills, CA 92 main (949) 716-0050; fax (949) 716-0
Proje	ct Numb	ح er: ح	02	7.((.16	0	Boring ID: PRD	Bench test boring (I
Proje	ct Name		ano	×	PP	ES -	Location: Cott	WRF rsol, NV
	ng Contr		TTN: NO					faich Ferninger
	ng Metho					Date	Started: 12/2/10 Total Depth: 4/6	
	ole Dia			-		Con	bleted: Surface Elev.:	TOC Elev.:
	ce Seal al (ft bg	s) Fro					То:	
Rema	- C	ollect	+ (20) 5 ga	Ø	t botter all bailer	forated zone ~ 20'-40' from boregale
Depth (ft)	Sample ID	Sample Time	Recovery %	Blow Count	Graphic Log	USCS Code	Material Desc	Wat
3 4 5 6 7 8 9 11 12 13 14 15 16 17 18 19 20 21						SP	Drown (7.5th Stel) poorly of trace fine SR-SA gravel, 1 dence, now plastic strong H sibrown (7.5th 1/2), trace fin ned coorse and, 75% u.f silt. T: trace fine gravel "Brown (7.5th 1/2) poorly grav dense, fine gravel dense, strong Hell Tran. now dense, strong Hell Tran. now	e - med gravely < 15% ne - fire soul, 10% ed soul, prace fine gravel colomed soul, ~ 70% s: 14, dry ad med
22 23 24 25 26 27 28							+ -7': brown (7.5(R-5/3) poorly gro med soul, 70% office - Fra wet med dense, 100 plas	del soud 101 5:14, -10% =

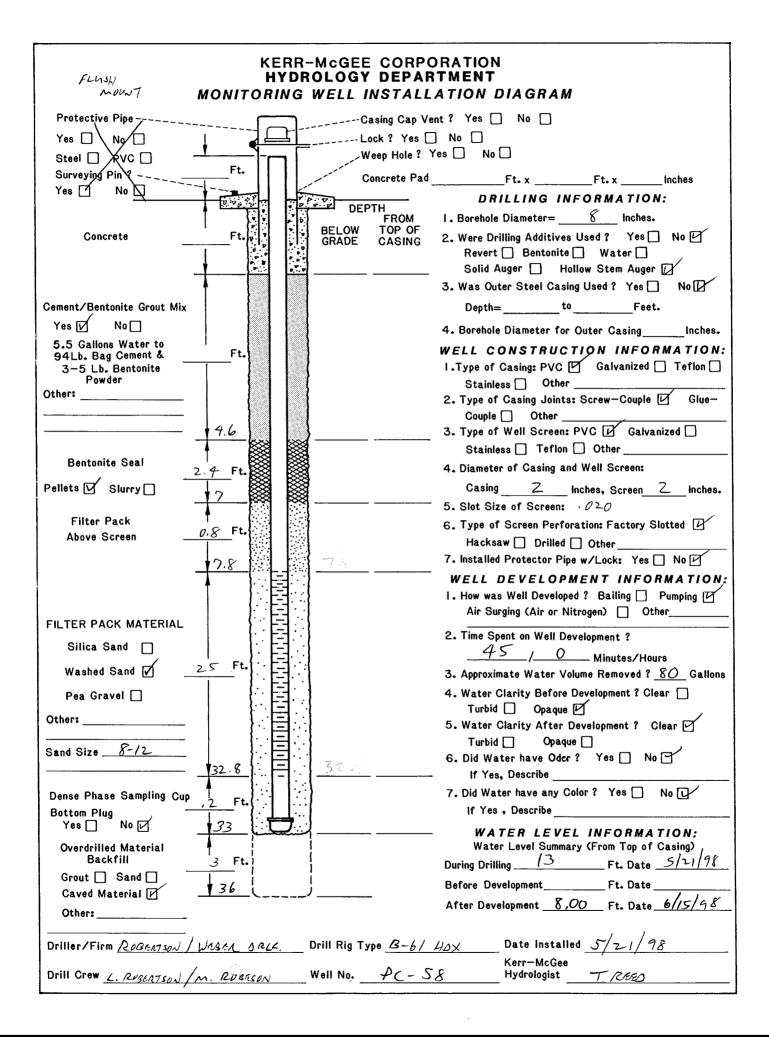
C	environm	thg:				Boi	ring Log	Northgate Environmental 24411 Ridg Laguna Hi main (949) 716-0050; fax (e Route IIs, CA 9 949) 716	Dri 926
Proje	ect Numl	ber: Z	202	7.	11.1	D	Boring ID: PRI Bench	test bating	ł	
Proje	ect Name	e: Tre	anox	-	DED	benen test	Location: CoH/L Herder Logged By: Patrice	DEF		
	ng Cont						Logged By: Patrico	· Ferninger		
Rem	arks:			9				0		
		не								Γ
Depth (ft)	Sample ID	Sample Time	Recovery %	Blow Count	Graphic Log		Material Description		Water Level	
31 32 33 34						-33'; frace fin	e - course R-Stgra	e (
35 36 37 38										
39 40 41 42 43								-		
42 43 44 45							7D@ 40'0	GS		
46 47 48 49							1001			
49 50 51										
- 52 - 53 - 54										
55 56 57										
58 59 60 61										
61 62 63 64										
65										
<u>: </u>	-					Pa	ge 2 of			L

KM SUBSIDIARY LOCATION BORING **KERR-MCGEE CORPORATION** C-18 HENDERSON NL Hydrology Dept. - S&EA Division KMC-UU UNIFIED BLOWS GRAPHIC LOG SOIL SAMPLE DEPTH REMARKS OR FIELD OBSERVATIONS PID SOIL FIELD IN FEET LITHOLOGIC DESCRIPTION PER (ppm) NO. DEPTH REC. 6" CLASS ه ای SILLY SAND IZD BRN San DRY WELL GRADED GRAVELS 5. 0.1 2 |....1 10 ?`? '0' ١ 6 ٥٠ 01 0.0.0.0. Q 15-Gm COLLECT 20 GROUNDWATER. SAMPLE AT 22 õ 6.5 Suns/grovel bin bong moist well geoded 0.00.00 25= ∇ 0.0 6.0.0.0 30 SAND/GROVEL GROUISY Gm BROWN WELL GRADEN SAT SILTY . 1.0 DATE DRILLED PAGE **GRAPHIC LOG LEGEND** ▼ Water Table (24 Hour) 4/2/98 1 of 2 DEBRIS ∇ Water Table (Time of Boring) DRILLING METHOD PID Photoionization Detection (ppm) HSA Identifies Sample by Number Sample Collection Method NO. TYPE ORGANIC (PEAT) EXPLANATION DRILLED BY SANDY CLAY WEBER SPLIT-BARREL ROCK CORE AUGER LOGGED BY GRAVEL J. CKanford THIN-EXISTING GRADE ELEVATION (FT. AMSL) CONTINUOUS SAMPLER NO RECOVERY SILTY CLAY WALLED TUBE LOCATION OR GRID COORDINATES DEPTH Depth Top and Bottom of Sample REC. Actual Length of Recovered Sample in Feet

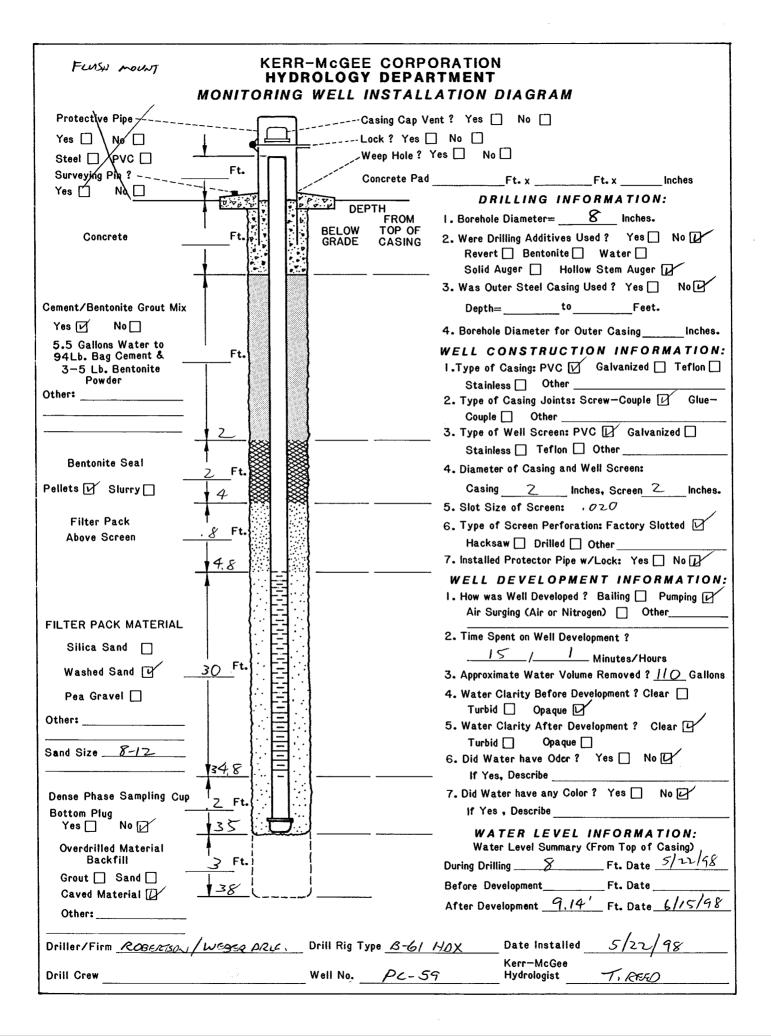
	ERR-MCGEE CORPORATION	KM SUBSIDI				LOCATION	••			BORIN		
H	/drology Dept S&EA Division	Kuc				+(EN	DERSO	SN 1	NU	NUMB	ER 40	-18
DEPTH IN			GRAPHIC LOG	UNIFIED SOIL	BLOWS PER	PID			SAMP	PLE		MARKS OR
FEET		NN	GRA	FIELD CLASS.	6'	(ppm)	NO.	TYPE	DEPTH	REC.	FIELD (DBSERVATIONS
			· • ·									~ .
	Sand/Graver BRW		0.0					X				59000 -
	- Sand / Grover Bew Well groves			Gm				Ĩ.			AT L	f2' _
1			0.:									
<u> </u> \$1-	-							allen av			toor in	Zeturns -
			- <u>o</u> .									
						<u> </u>						_
	4					 						-
50-	SAND BRU F-CRS Gr	Sm		< 1				$\overline{\nabla}$			T/MUI	
	14" grover were groon		10/2	SW				4				54 -
T/mc	SAT LOUSE SLISILAY	/	N.		ļ						<u> </u>	_
	- SILTY CLAY ED BRN .		XZA	4				Д				-
-	- SM FINE SAND & SMO	u/										
	GRAVELS	_/ /										-
	SILTY CLAY GREENISH	• 1				<u> </u>					DRILL	TD 53' -
	W/ tan to begin var	214.										
-	BLOCKLY											
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	4					<u> </u>						-
	-											-
	1											
	Water Table (24 Hour)					GRAPHIC				DATE DRILLED	8	Z of Z
	☑ Water Table (Time of Borin PID Photoionization Detection (g)				CLAY		DEBRIS Fill	5	PRILLING MET	-	
	NO. Identifies Sample by Numb YPE Sample Collection Method					SILT		HIGHLY ORGANIC	(PEAT)	HSA	٢	
10		רופרן				SAND		SAND' CLAY		DRILLED BY	SER	
ANA	SPLIT- BARREL AUGER	F	ROCK CORE		1				1	OGGED BY	<u>,</u>	
EXPLANATION			10			GRAVEL		CLAYE SAND		-	POWFOR	ON (FT. AMSL)
	WALLED CONTINUOU TUBE SAMPLER	2	NO RECOVE	RY		SILTY CLAY			'	LAIS HING GR/	NUC ELEVAII	UN (FI, AMOLI
	DEPTH Depth Top and Bottom of S REC. Actual Length of Recovered	Sample	n Faat			CLAYEY SILT				LOCATION OF	R GRID COOF	DINATES
	REC. Actual Length of Recovered	a sumple i	nieer									



	REMINICULE CONFORMION	KM SUBSIDIARY			LOCATION				BORING	
L	Hydrology Dept S&EA Division	KMCLU		r	HEND	ERSO	N,	N/		R PC-58
DEP IN FEI	LITHOLOGIC DESCRIPTION	1 1 (0)	UNIFIED SOIL FIELD CLASS.	BLOWS PER 6'	PID (ppm)	NO.	SO EVPE	IL SAMI DEPTH		REMARKS OR FIELD OBSERVATIONS
2	BERM : SAND W/ GRAVEL	21 5						·····		-
5	SAND W/ SILT; MED. BR SLI! MUIST; OCL. GRAVEL		SM							ר ר ער ר
10			GM							
13							A S. S. S. Market Statistics			
15							and the second se			
20			5~							
25	GRAVEL ZONE C 26-28		<i>cm</i>							
34	 		<u> </u>							
35	U. SLI, PLASTIC; MODOY CR	uis H-WHITE v≤K	CL-		<u> </u>	1		35	1,5	
36	- TD 36'		\	-	 			<u>36.5</u>		
F	Water Table (24 Hour)	I	<u> </u>	6	FRAPHIC I		GEN		ATE DRILLED	PAGE
Z	ValueValueValueValueValueVWater Table (Time of Boring)PIDPhotoionization Detection (ppNO.Identifies Sample by NumberTYPESample Collection Method) pm) r			CLAY SILT				5/21/9 DRILLING METH 145 DRILLED BY	IOD
EXPLANATION	SPLIT- BARREL AUGER				SAND GRAVEL				<u>ω εβ</u> OGGED BY -T, k	
ŭ	THIN- WALLED SAMPLER		۲Y		SILTY CLAY			F		ELEVATION (FT. AMSL)
	DEPTH Depth Top and Bottom of Sa REC. Actual Length of Recovered	لاــــا Imple Sample in Feet			CLAYEY SILT					GRID COORDINATES EAST OF PC-S6

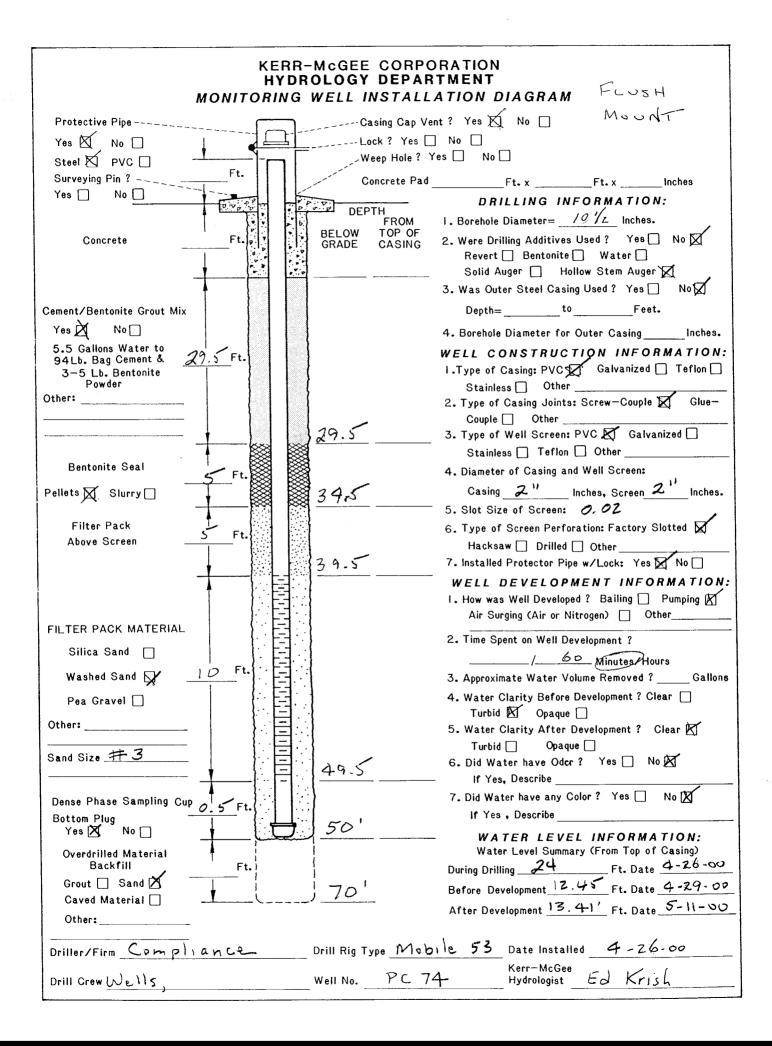


		R-McGEE CORPORATION	KM SUBSIDIA				LOCATION				BORIN	G G
	1	rology Dept S&EA Division	Kma				<u> +ENI</u>	VFLSC				ER PC-59
DEP IN FEI	1	LITHOLOGIC DESCRIPTIC	N	GRAPHIC LOG	UNIFIED SOIL FIELD CLASS.	BLOWS PER 6'	PID (ppm)	NO.	SO IYPE	DEPTH	E REC.	REMARKS OR FIELD OBSERVATIONS
z 5		BERM: JAND W/ GRAVE SAND W/ SILT; OCC. GR MED. BROWN; WELL- GRA SCI'MOIST GRAVEL @ 3-4'	AVELS	0	Sm 							
15 20		SAND AS ABUR; SATURAT OCC. GRAVEL	¢0;		Sm							
2 < 30		SANOY SILT; LT. BEIGE; BCC FINE-MED, SAND	SATURATUS;									
3 . 35	. 1	SILTY CLAY; CLAYEY-SILT GREGO-BELAG; U. SLI. PLAS	; MED-		CL-		 					
3		MUDOY CRE			ML							
		70 38'						1	M	38 39,5	1,4'	
	_	Water Table (24 Hour)					RAPHIC I				re drilled	PAGE / of /
NO	_√ PID NO TYP	 Identifies Sample by Number 	pm)						HIGHLY ORGANI		LLING METH	
EXPLANATION		SPLIT- BARREL AUGER	RC	DCK DRE			SAND GRAVEL			10	GGED BY	BER DRIG. REED
EX		THIN- WALLED TUBE			RY		SILTY CLAY					ELEVATION (FT. AMSL)
		PTH Depth Top and Bottom of So C. Actual Length of Recovered	ample Sample in	Feet			CLAYEY SILT					GRID COORDINATES WEST OF PC-56



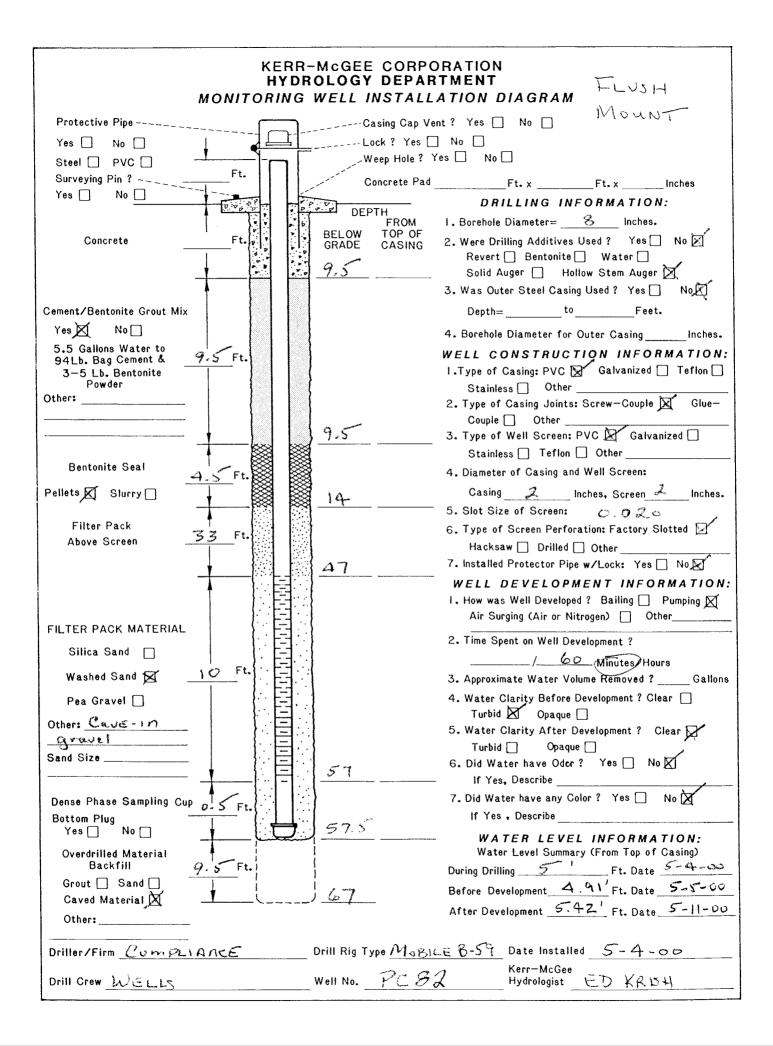
	RR-McGEE CORPORATION drology Dept S&EA Division	KM SUBSIDIARY			Hend	erso	n	NV	BORING	er PC 74
DEPTH IN FEET	LITHOLOGIC DESCRIPTIO	GRAPHIC	2 UNIFIED SOIL FIELD CLASS.	BLOWS PER 6'	PID (ppm)	NO.	TYPE SC	DIL SAMPL DEPTH	E REC.	REMARKS OR FIELD OBSERVATIONS
	0-8 GRAVEL W/ Sd, yell orange. 30% gravel + boulde 2' diam. volcant 60% vc-vf SA Sd 10% Silt 8-15 SILY SAND W gravel. Inc in silt 25%, gry bry. SC vf-vc, SA-A. Gri w/minor silt. gry b volc grav. to 2" Sd vc-f SA-A ma 10-15% silt 21-24 Cly SAND grn, Sd f-vc w com (40%) clau 24-51 Pen Gravel vc-f Sd matrix. gry. St. Slty 25% sd 70% pengy 27-28.5W/ com cobble boulders 37-48 boulder 27 (thin) Scattered throughout	rs to 00000000000000000000000000000000000	W S S S S S S S S S S S S S S S S S S S							damp@ 7' = 12.45' = 4.29.00
 ₽I	 Water Table (Time of Boring Photoionization Detection (p 	g) ipm)	<u>- 1</u>		GRAPHIC		DEE	BRIS DR	TE DRILLED	HOD 1 of Z
					SILT SAND GRAVEL SILTY CLAY SILT		ORG/ SAN CLA	ANIC (PEAT) DR NDY AY ND ND EX	GGED BY	A pliance KRISH DE ELEVATION (FT AMSL) GRID COORDINATES

KM SUBSIDIARY LOCATION KERR-MCGEE CORPORATION BORING NUMBER PC 74 Henderson NV Hydrology Dept. - S&EA Division KMC LLC UNIFIED BLOWS DEPTH GRAPHIC LOG SOIL SAMPLE REMARKS OR FIELD OBSERVATIONS PID SOIL IN FEET LITHOLOGIC DESCRIPTION PER FIELD ΥPE (ppm) NO. DEPTH REC. 6" CLASS. 000 SCREENED 40'-50' 0.0.0.0 GW WTR SMPL 4-28-00 0.0 0 PH 7.3 TDS 7100 51 . 51- 56 SAND, m-VC, SA-SR, grn gry, hard. SI. SW sity (10%). w/ 10% granules 56 56-70 sity sdy CLAY, Muddy Creek @ 56' gen gry area brn, mutaled. 60 _ Calcareous, sticky, drills slow. w/ 5-15% vf-ma CL-ML sand in matrix. Contains 10% c-vc-gran sized caliche nodules dissen. throughout TD 70' 7 GRAPHIC LOG LEGEND DATE DRILLED PAGE **T** Water Table (24 Hour) 4-26-00 Z of Z DEBRIS V Water Table (Time of Boring) Photoionization Detection (ppm) Identifies Sample by Number DRILLING METHOD PID HSA NO HIGHLY ORGANIC (PEAT) TYPE Sample Collection Method EXPLANATION DRILLED BY SANDY CLAY Compliance SAND SPLIT-BARREL ROCK CORE AUGER LOGGED BY Å GRAVEL KRISH E THIN CONTINUOUS SILTY CLAY EXISTING GRADE ELEVATION (FT AMSL) NO WALLED SAMPLER RECOVERY TUBE CLAYEY SILT LOCATION OR GRID COORDINATES DEPTH Depth Top and Bottom of Sample REC. Actual Length of Recovered Sample in Feet

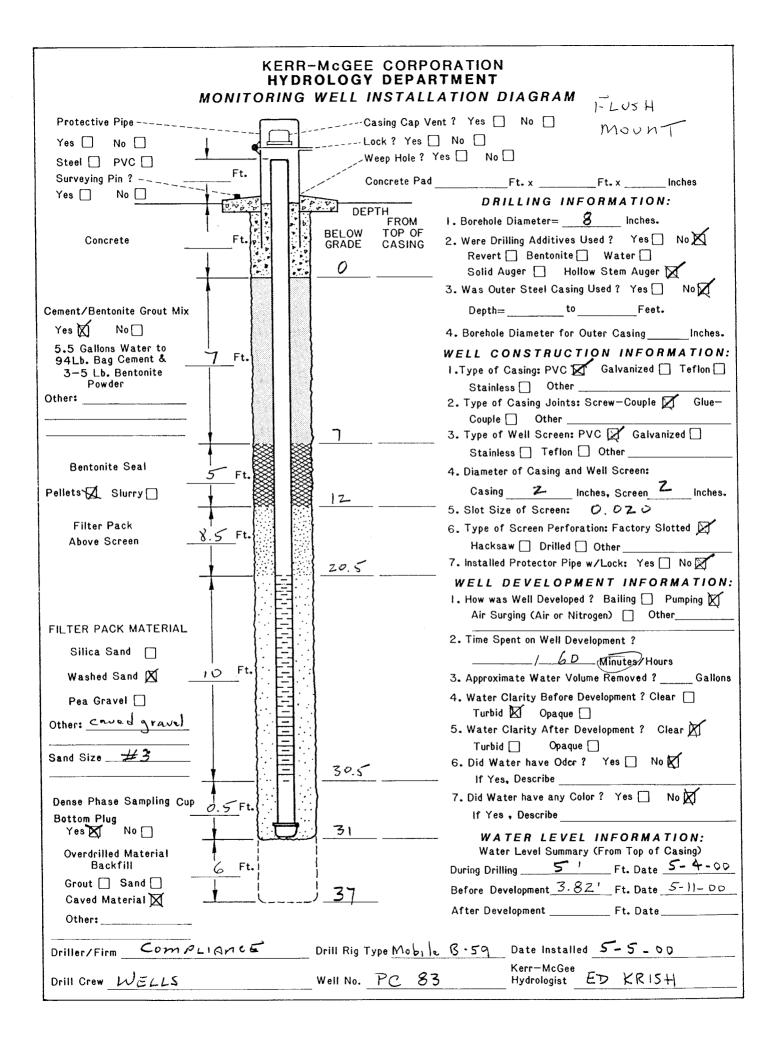


SOIL BO	RING LOG KM-5655-B								
	RR-McGEE CORPORATION drology Dept S&EA Division	KM SUBSIDIARY	LLC		HENT	>ERS	oN, N'		G PC 82
DEPTH IN FEET	LITHOLOGIC DESCRIPTIO	Z SRAPHIC LOG		BLOWS PER 6'	PID (ppm)				REMARKS OR FIELD OBSERVATIONS
	0-3 disturbed &	11.*	SM-						@z'damp
	3-12 sity gravelly dk brn, 20% silt, e granules & pea gravel of volc. 40% vf-vi SA-SR	40% (3R)	SM- GM						<u>7 @ 5 '</u>
12 15 - - - - - - - - - - - - - - - - - -	12-15 sdy cly SII dkbrn, 15-25% vf.m, isd, 20% clay, 10% Volc granules. stic 15-20 sity sdy GRA dkbrn, SR-SA, 20% 25% vf-vc sd, SA-S <u>20-30</u> sity gravelly dk brn, 20% silt, 40	$SA \cdot SR$ Sm VEL, VEL, Silt, R SRND Volc.	ML GM						
- 25	granules to peagran SR; vf-vc, SR-SA sc		SM- SM-	1					
33 35-	30-33 sdy sity GR Uk brn, 30% SA-SR, 20% silt, 50% volc 1/s gravel to 2-3", SR 33-39 sity gravel as above Q 38'-39' gravel zone SR up to 3" diam 39-46 sity SAND, to	vf- 2 sd 00 v/ minor 00 v/ minor 00 v/ sAND 00 v sAND 00	GM GM 5000000000000000000000000000000000						
<u>-7</u> F N	Water Table (24 Hour) Water Table (Time of Borin. PID Photoionization Detection (provide the structure of the struc	g) ppm)			GRAPHIC CLAY SILT		GEND DEBRIS FILL HIGHLY DRGANIC (PEAT)	DATE DRILLE 5-4-0 DRILLING ME	o 1 of Z
	SPLIT BARREL WALLED TUBE DEPTH Depth Top and Bottom of S REC. Actual Length of Recovered	RECC	VERY		SAND GRAVEL SILTY CLAYY SILT		SANDY CLAY CLAYEY SAND	Cor Logged by E1 Existing gr	R GRID COORDINATES

KERR-MCGEE CORPORATION Hydrology Dept S&EA Division						LOCATION	0-1			BORING NUMBER PC82		
	drology Dept S&EA Division	- ML		UNIFIED	RI OWA	HENDE	K201					
DEPTH IN FEET	LITHOLOGIC DESCRIPTIO	N	GRAPHIC LOG	SOIL FIELD CLASS.	PER 6'	PID (ppm)	NO.	17PE	DIL SAN		REMARKS OR FIELD OBSERVATIONS	
	SA, 30% silt in 70%	4-4~				_						_
-	sd w/ minor c-vegi	raing		SM		<u> </u>						-
	sticky calcareous			2101								
45-						<u> </u>					-	\neg
	46-50 sity gravelly	SAND.	0.0	sm-	+	· ····································						
-	dk brn, as above	U Uj	0.0	GM								_
<u>50</u>			: -1:	GW								
	50-52 cly, sity sAN	P, 17.		SM- SC								
52 -	red brn + grn gry. Si vf-fg sd wi zoz. cla	9-5R	1.0	SM-								-
-	silt. Com sm calich	27 4 50%										
	Inodules ; calcareous		0.00								-	_
56 1		SAND	$\overline{}$	ļ			m.	bd	dy C	k @ 56		
-	f-c SR-SA w/ ZoZ silt 3015 volc + 15 pebbles f	oz"		1								
60 -	56-67 sty CLAY,		X	0.							_	
-	grn yellow, sticky	•	V_{λ}									_
			K/									
-			1/			-						_
65 -			Λ	1							-	_
67 -			1/	1					ļ			
-	TD 67'											
											-	
-												_
. –												_
-												_
-												
-	_											
J	Water Table (24 Hour)		<u> </u>	.1	(GRAPHIC	LOG LE	GE	I ND	DATE DRILLEI		
2						CLAY		DEB Fill	RIS	5-4-		
	 Photoionization Detection (p Identifies Sample by Number PE Sample Collection Method 					SILT			LY NIC (PEAT)		5A	
ATIO	71				I	SAND				DRILLED BY	LIANCE	
	SPLIT. BARREL	R	OCK ORE			GRAVEL				LOGGED BY		
EXP			10		1	GRAVEL SILTY CLAY		JAC	טי		KRISH ADE ELEVATION (FT AMSL)	
	TUBE		ECOVE	κĭ		CLAY CLAYEY SILT					R GRID COORDINATES	
	EPTH Depth Top and Bottom of S REC. Actual Length of Recovered		n Feet		017	I SILT						

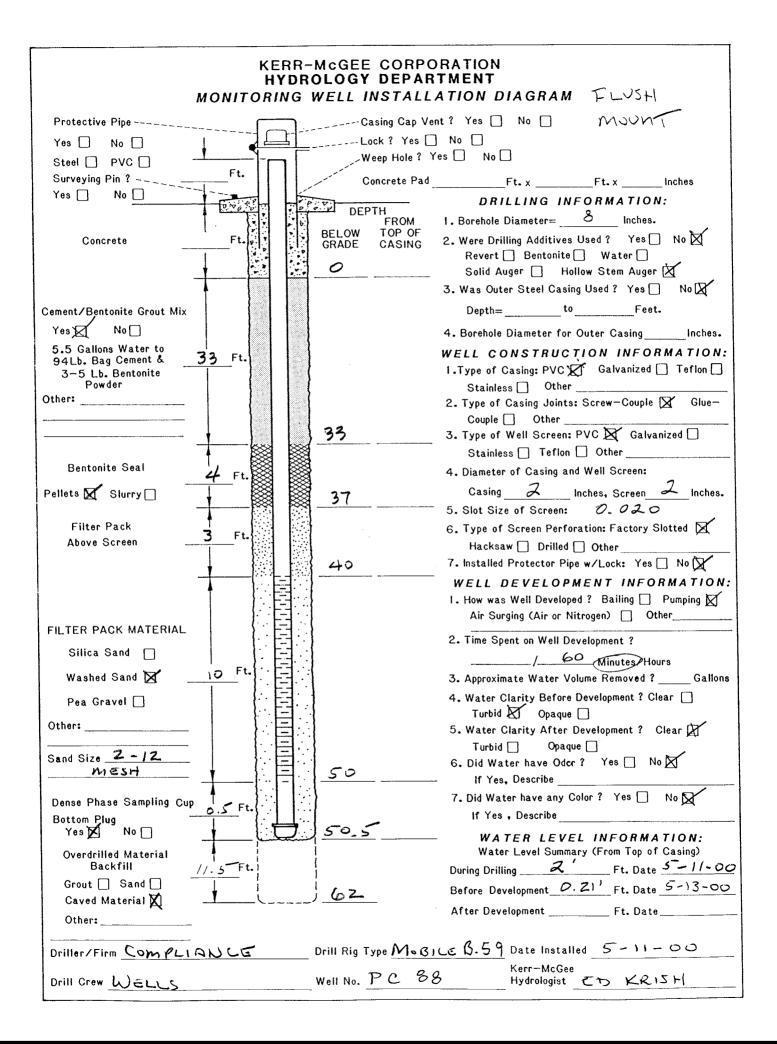


OIL ROH	NG LOG KM-5655-B				,							
	R-McGEE CORPORATION rology Dept S&EA Division	KM SUBSIDIARY	LLC		LOCATION	DER:	50	N.NV	NUMBER PC 83			
DEPTH IN FEET	LITHOLOGIC DESCRIPTIC		O UNIFII	D BLOWS PER 5. 6'	PID (ppm)		SC Bd	DEPTH	REC.	REMARKS OR FIELD OBSERVATIONS		
	PC 83 is 11' N pf PC 82. See log for Pa for lithology 37' TD	1								<u><u>v</u> @ 5' 5-4-00</u>		
	 Water Table (Time of Borin Photoionization Detection (Identifies Sample by Numb 	ppm) er IS IS Sample	e Dvery		GRAPHIC CLAY SILT SAND GRAVEL SILTY CLAYEY SILT		DEI FILI HIGH ORG SAI CL/	BRIS L D ANIC (PEAT) ANIC (PEAT) AY AY AY ND E	COM COM COGGED BY ED EXISTING GRA	so 1 of 1		

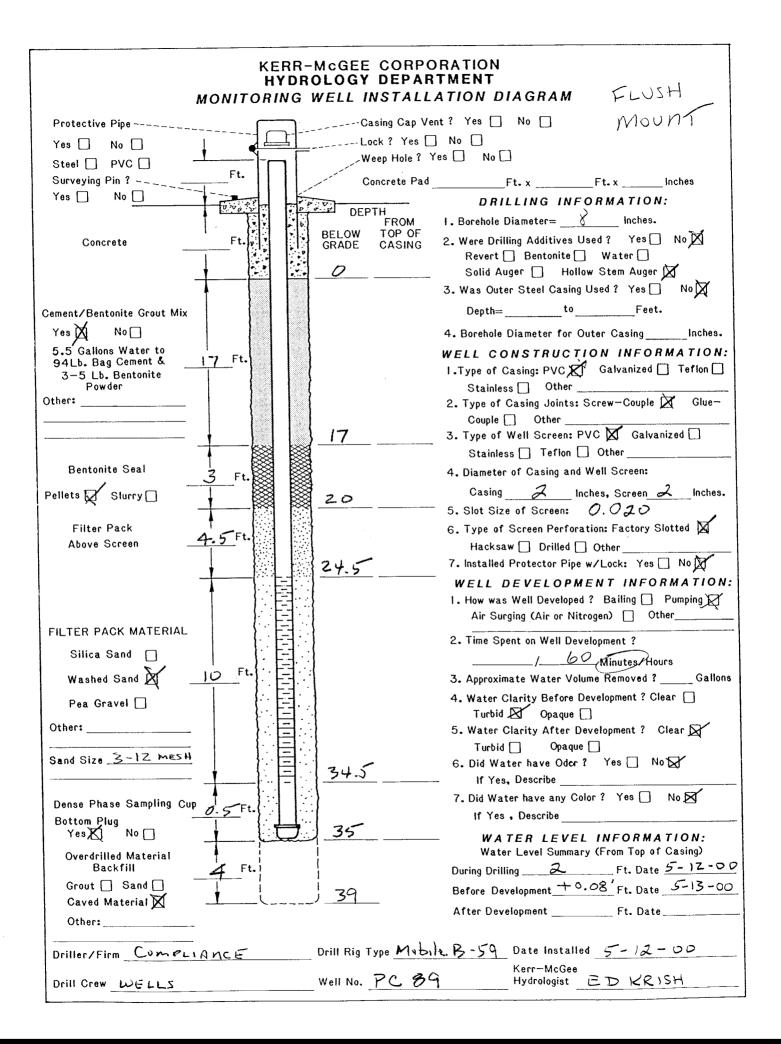


SOIL	L BORING LOG KM-5655-B											
	KERR-McGEE CORPORATION	KM SUBSIDIARY	LC		LOCATION	ÉAN		, NV	V BORING PC 88			
	Hydrology Dept S&EA Division	1	UNIFIED	DIONE					<u>т</u>	· · · · · · · · · · · · · · · · · · ·		
	N LITHOLOGIC DESCRIPTIC	GRAPHIC LOG	SOIL	PER	PID (ppm)	 		DIL SAMPL		REMARKS OR FIELD OBSERVATIONS		
FE	ET		CLASS.	6'	(ppm)	NO.	TYPE	DEPTH	REC.			
	- 0-12 Sdy GRAVE									Jampeo'		
	pale brn ('5YR 5/2)). 10% 000	1							Vez'		
] silt, 30% sd (SA-SR	1, vt - 000	GW									
5	- vc) and 60% vol		GN		 							
	- gravel (SA-SR, up-	to3" 000								-		
	diam.	000						-				
	_	000	,							_		
10)	000										
	-	000										
1	- 12 - 51 sty gran	velly 1:0										
			-									
112	(10YR 6/2). Var. si	1+										
	20-40%. 20-30%	pea ::								_		
	- gravel to 3/4" (volu		:							-		
Zo	Sand SA-SR vf-	VC 0	5M-									
	12-21 10-20% 51	Hγ :11;	-GM							_		
	matrix									_		
1.			•							_		
K,	- <u>21-51</u> com siltin - 30-40%	n matrix :	•			i						
		• • •	-									
20	0- 27-33 gravel zone , pebbles to 3". Var.	$\omega/$ []: c										
			1		<u> </u>							
	-32-33 v. hard, slow - abu caliche cema	drilling	,. 									
	abu coliche cemu	ent !!!	•							_		
35	5											
	-		•		-							
	37-51 Var. amts	4	5									
	= 37-51 Var. amts of gravel (pebbles to up to 50%	2") 00	- I		<u> </u>					-		
F				6	RAPHIC I	l OG LE	GEI			PAGE		
	Water Table (24 Hour)	a)			CLAY			Z	5-11-			
	PID Photoionization Detection (p NO. Identifies Sample by Numb	opm)						Y NIC (PEAT)	LLING METH	5A		
lon	TYPE Sample Collection Method								LLED BY			
EXPLANATION	SPLIT- BARREL AUGER	ROCK			SAND			10	GGED BY	NPLIANCE		
XPLA					GRAVEL		SAN	1	ED KRISH			
μ	WALLED TUBE		RY		SILTY CLAY			EXI	STING GRAU	DE ELEVATION (FT AMSL)		
	DEPTH Depth Top and Bottom of S REC. Actual Length of Recovered				CLAYEY SILT			LO	CATION OR	GRID COORDINATES		

SOIL BORING LOG KM-5655-B LOCATION KM SUBSIDIARY BORING **KERR-McGEE CORPORATION** PC 88 NUMBER HENDERSON, NV KMC LLC Hydrology Dept. - S&EA Division GRAPHIC LOG UNIFIED BLOWS SOIL SAMPLE DEPTH REMARKS OR FIELD OBSERVATIONS PID SOIL FIELD PER IN FEET LITHOLOGIC DESCRIPTION (ppm) Ξd λ. DEPTH NO. REC 6'' CLASS 0.00 NOTE -5M Most likely this unit (0) 13 a series of fluvial fining - upward sediments (0) 1 a strict (0) GM 45 from gravels to silts 10-0 0:0 51 MC CSI' 51-62 sty CLAY, grn gry (5648/2) and yell gry (548/1) 55 CL 60 6Z 62' TD DATE DRILLED PAGE GRAPHIC LOG LEGEND T Water Table (24 Hour) Z of Z 5-12-00 CLAY DEBRIS ∇ Water Table (Time of Boring) DRILLING METHOD PID Photoionization Detection (ppm) HIGHLY ORGANIC (PEAT) HSA NO. Identifies Sample by Number Somple Collection Method TYPE DRILLED BY EXPLANATION SANDY CLAY SAND COMPLIANCE ROCK CORE SPLIT BARREL AUGER LOGGED BY GRAVEL CLAYEY SAND ED KRISH THIN EXISTING GRADE ELEVATION (FT AMSL) CONTINUOUS SAMPLER NO RECOVERY WALLED TUBE LOCATION OF GRID COORDINATES DEPTH Depth Top and Bottom of Sample Actual Length of Recovered Sample in Feet REC.

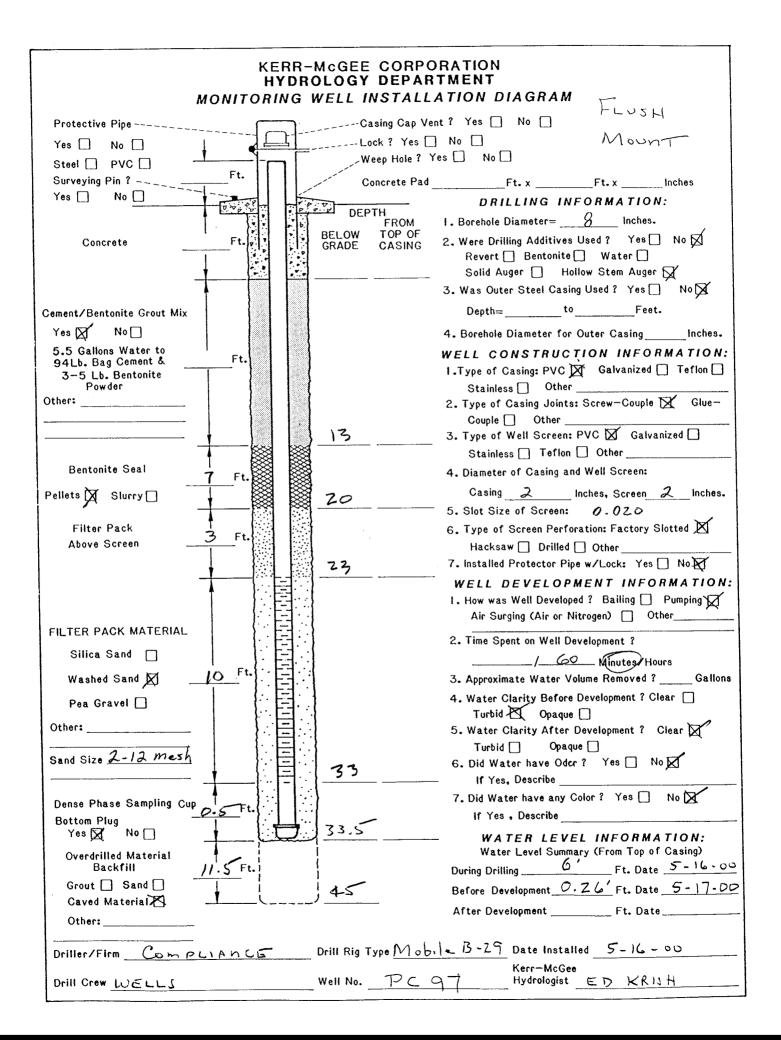


SOIL BORING LOG KM-5655-B KM SUBSIDIARY LOCATION **KERR-MCGEE CORPORATION** BORING HENDERSON, NV PC 89 KML LLC NUMBER Hydrology Dept. - S&EA Division GRAPHIC LOG UNIFIED BLOWS DEPTH SOIL SAMPLE PID SOIL FIELD REMARKS OR IN FEET LITHOLOGIC DESCRIPTION PER FIELD OBSERVATIONS (ppm) ŕ DEPTH 6'' NO. REC. CLASS VCZ' PC 89 located 7' east of PC 88 See log of PC 88 for lithology 5 10_ 20 25 30 35. 39 TD 391 DATE DRILLED PAGE **GRAPHIC LOG LEGEND .** Water Table (24 Hour) 5-12-00 of) CLAY DEBRIS ∇ Water Table (Time of Boring) DRILLING METHOD PID Photoionization Detection (ppm) HSA NO. TYPE Identifies Sample by Number HIGHLY ORGANIC (PEAT) Sample Collection Method DRILLED BY EXPLANATION SANDY CLAY SAND COMPLIANCE SPLIT BARREL ROCK Core AUGER LOGGED BY GRAVEL CLAYEY ED KRISH THIN EXISTING GRADE ELEVATION (FT AMSL) CONTINUOUS NO SILTY CLAY WALLED TUBE SAMPLER RECOVERY CLAYEY SILT LOCATION OR GRID COORDINATES DEPTH Depth Top and Bottom of Sample REC. Actual Length of Recovered Sample in Feet

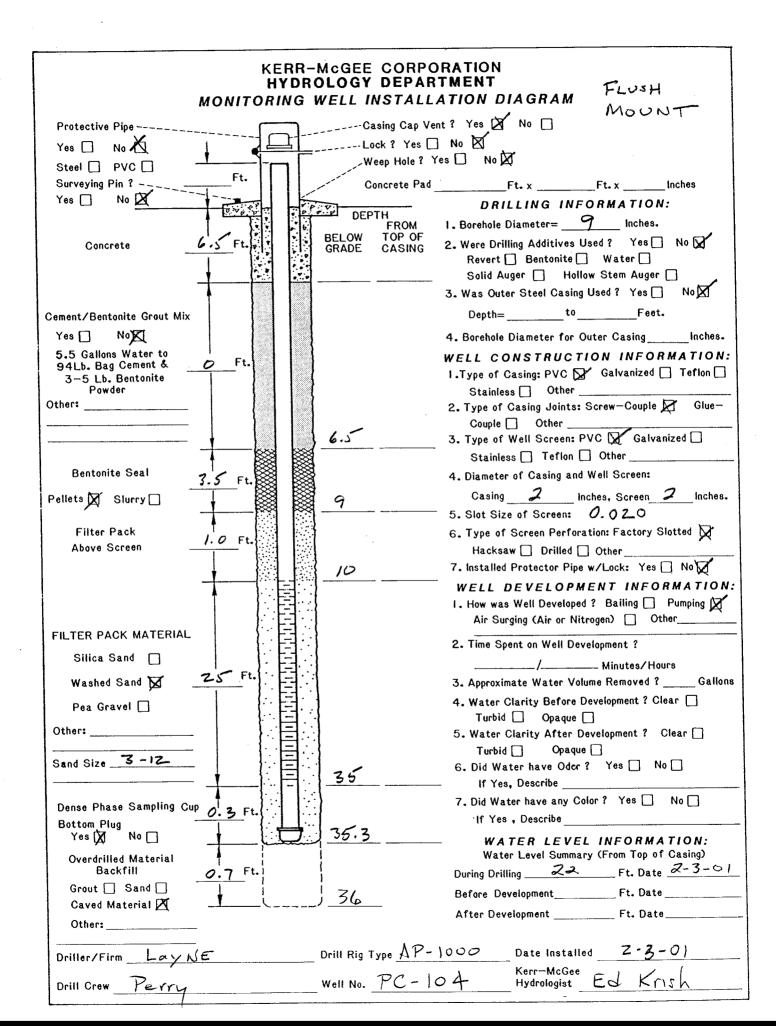


SOIL	SOIL BORING LOG KM-5655-B											
KERR-McGEE CORPORATION KM SUBSIDIARY Hydrology Dept S&EA Division KM C LLC					LOCATION HENI	DERS	nxl		BORING PC97			
			()	UNIFIED	RI OWS						· · ·	
DEPT IN	LITHOLOGIC DESCRIPTIC	м	GRAPHIC LOG	SOIL FIELD	PER	PID (ppm)	NO.	LYPE	DEPTH	REC.	REMARKS OR FIELD OBSERVATIONS	
FEE		~ 1	Ö	CLASS.	6'		NO.	2				
	0-5 BERM materi		10.0	,		·					_	
	- brn sity gravelly	SAND	0	52							dampe3'	
			1.0								_	
5 -	- 7.2 11		.0.1									
	- 5-20 Sity grave - SAND, pale brn		10-									
	SAND, pale orn	(5485/2)	0	,		·				-	_	
	granules and sm p up to 1" Jiam Sand 10 vf-vc, SA										_	
10.	granules and sm p	cbbles		5. /								
	up to 1" diam		0.	SW								
	- Sand 10 vf-vc, SA	-sr				<u> </u>					_	
. /	-										-	
15			0									
			•								_	
	_					<u> </u>						
20	_		0	•								
100	20-25 5144 SANT	5 w/	1.1-1									
	minor gravel. po	Ja yell	0.	5M								
	brn (104R 6/2). S.	H up to									_	
25	= Z5%, gravel (gran.	+ pen _	[]0]	•				<u> </u>				
	- size up to 20%, Sa		0 0 0			<u> </u>					-	
	abrue rf-vc, SR	- SA .	00	•								
				。 。								
30	- Z5-36 sity sdy pale yell brn (IOVR	6/2).	100	GW		<u> </u>						
	- 7=7- 514 257- vf-1	16 .5A-5	RUJI			<u> </u>		ŀ			-	
		are and	0.01	0							-	
	- and pubbles to 2" d	ham	000	01		—						
	- ond publies to 2"d locally com caliche - 36-42 silty Sf	cement	00.	0		<u> </u>						
36	36.42 silty SA	ND		;	~							
	- Pale yell brn (10 yr	6/2)		SM								
	- bimodal: vf-fg w/					<u> </u>					-	
	✓ Water Table (24 Hour)					GRAPHIC	LOG LI	EGE	ND	DATE DRILLE		
	Water Table (Time of Borin	ng)				CLAY		DE	BRIS	5-16.		
z	PIDPhotoionization Detection (NO.Identifies Somple by NumberTYPESample Collection Method	ppm)			1	SILT			ILY		HSA	
EXPLANATION			ROCK		1	SAND			NDY AY	Com (DLIANCE	
XPLAP			CORE		1	GRAVEL	-] CL SA	AYEY ND	ED	KRISH	
ш Ш	WALLED TUBE	us 🚺	NO RECOV	ERY		SILTY CLAY					ADE ELEVATION (FT AMSL)	
	DEPTH Depth Top and Bottom of REC. Actual Length of Recovere	Sample d Sample	in Fee	t .		CLAYEY SILT]		LOCATION O	R GRID COORDINATES	

KERR-McGEE CORPORATION KM SUBSIDIARY Hydrology Dept S&EA Division KM C LLC				HENDI	ERSOI	J	NV	BO NU	PC 97			
DEPTH				UNIFIED SOIL FIELD	PER	PID (ppm)		so	IL SAM	1		REMARKS OR FIELD OBSERVATION
FEET FZ F3 F3 F3 F3 F3 F3 F3 F3 F3	C-VC, SR, Sand. Z Silt in matrix. Calc 42-43 Silty gravelly Pale yell brn. Gravels 3/4" Jian W/minor cell cement, calcarcous 43-45 cly sdy Silt It grn gry (5648/1) clay in matrix, 10-0 VI-fz sand. Calca W/mmor sm. calicher TD 45'	sand sand sand up to che T , 10-zozz zozz zozz reous,	1	SM SM ML- CL	6'		NO.		DEPTH			ense + dry AC @ 43
, - 	Water Table (24 Hour) Water Table (Time of Borin PID Photoionization Detection (NO. Identifies Sample by Numb YPE Sample Collection Method SPLIT- BARREL	ppm) ber)CK DRE			GRAPHIC CLAY SILT SAND GRAVEL		HIGH FILL ORG SAI	BRIS ILY ANIC (PEAT)			1
	THIN- WALLED TUBE DEPTH Depth Top and Bottom of REC. Actual Length of Recovere	RE Sample	COVI		Ø	SILTY CLAY CLAYEY SILT]		EXISTING	G GRADI	RED COORDINATES



KERR-McGEE CORPORATION KM SUBSIDIA Hydrology Dept S&EA Division KM Y			تل			LOCATION Hen	der:	50,	n, NV	BORIN	
DEPTH			UH0	UNIFIED SOIL		PID		sc	DIL SAMI	PLE	REMARKS OR
IN FEET	LITHOLOGIC DESCRIPTIC	м	GRAPHI LOG	FIELD CLASS.	PER 6'	(ppm)	NO.	ТҮРЕ	DEPTH	REC.	FIELD OBSERVATIONS
	0-6 Bern Mater	ial	/								
_	sdy, gravelly Mi	X									
-											
6-			/						ana an		
	6-21' SAND, gran	velly	0.0			<u>.</u>				-	
_	\$ silly. Brn (5YR5/	4)·	10								
10 -	10-20% siltin sd ma		0.1								_
/	f-cgw/mmorrcg, 51	、∼SR,	0								
	20-30% SA-SR, Vole		0.0	SW							
_	gravel to 3/4" w/1	ه دم ۱۱۲				 					
15 <u> </u>	thin zones to 2". A calcareous.	lon-	0								
	4 .		010			_					
	<u>6-12</u> com gravel to	2"	0.0	•		<u> </u>					
_			1.1.2.9								
			00	:							-
21 -						10		-			
	21-35 GRAVELE	Sdy	000			<u> </u>	1				damp@Z1'
	GRAVEL, interbedde (5YR5/4). Volc clas-		0.00	2							wet @ zzi
25 -	1" mere of locally to	5".	0.0								
-	sA-SR, contains var	. amts	00.0	GP/		 					
	of vf-vc, SA-JR SI	d. +r		GM	ι						
-	<u>z3-z6</u> com lg grave	11.4"	0.0								
30	79-20 ElL. 070.11.11	SAND	100	1) - 1							
-	29-30 sity gravell. vf-vc w/ 20-30%	Isilt									
			0.0			<u> </u>					
	34-35' com lg grav.	el to 5'									
35-			0.0.0								
36.	35-36' CLAY, S		122	QCL							MC@35'
-	gryyellgrn (56y) 10-20% silt in mati	riz, nor									
	calcareous. Tr-s	pgypsu.	m								
<u> </u>	TD @ 36'				1	 GRAPHIC		FCI			D PAGE
										Z-3-0	01 1 of 1
	Z Water Table (Time of Borin Photoionization Detection (ppm)			1	CLAY			BRIS L	DRILLING ME	
	IO. Identifies Sample by NumE YPE Sample Collection Method	ber				SILT	$\langle \rangle$	HIG ORC	hly Ganic (peat)	YER DRILLED BY	CUSSION
						SAND		SA CI	NDY AY	LAV	NE
EXPLANA	SPLIT- BARREL AUGER		ROCK CORE			GRAVEL			AYEY	LOGGED BY	
XPL				1		<u> </u>	א א ר	ND		1 <rish< td=""></rish<>	
	WALLED CONTINUOL TUBE SAMPLER		NO RECOV	ERY		SILTY CLAY				EXISTING GRADE ELEVATION (FT AMSL	
	DEPTH Depth Top and Bottom of REC. Actual Length of Recovere	Sample d Sample	in Fee	t	BI	CLAYEY SILT]		LOCATION O	R GRID COORDINATES



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	RR-McGEE CORPORATION drology Dept S&EA Division		_L C		LOCATION	NDE	K.	50N	BORING NUMBER PC 116 R		
DEPTH IN FEET	LITHOLOGIC DESCRIPTION		RAPHIC	UNIFIED SOIL FIELD	PER	PID (ppm)	NO.	SC JAFE	DIL SAMPL	1	REMARKS OR FIELD OBSERVATIONS
	0-10 GRAVEL, S and SAND, grave interbedded. Min sity layers. Brn 50-80% gran - Z 10-30% silt in Sdy mi zo-50% vf-vc, SA	lly - or thin "peb. atrix	0.00	GW/ SW	6'			1	DEPTH	REC.	damp@1'
-	10'-18' SAND, brn, vf-cg, SA 10-3070 silt in Ma locally com. sd-sh caliche nodules	sity trix ze		SМ							
	18-20 SILT, Sdy, gr Com Calche nods, 20-30 <u>ZO-Z7</u> SAND, SIH brn. vf-mg w/mi c-vc. ZO-30% Si matrix	~,1+.		ML SM							Jamp - WTR@Zo' -
-	27-49 GRAVEL W/minor gravelly s and silty sand. Pa (Series of fining-up 70% volc+1s. gran cobbles. 20-30% f and thin layers w/ silt in sdymatrix 27-38 peagravel	seq.)	0.0.	GP/ GM							
	 Water Table (Time of Boring Photoionization Detection (p Identifies Sample by Number 	er B S N N Re N Re	OCK ORE O ECOVE			GLAY SILT SAND GRAVEL SILTY CLAY SILTY SILT		DEB FILL NGH ORG/ SAN CLA		GGED BY E	5 to 1 10.

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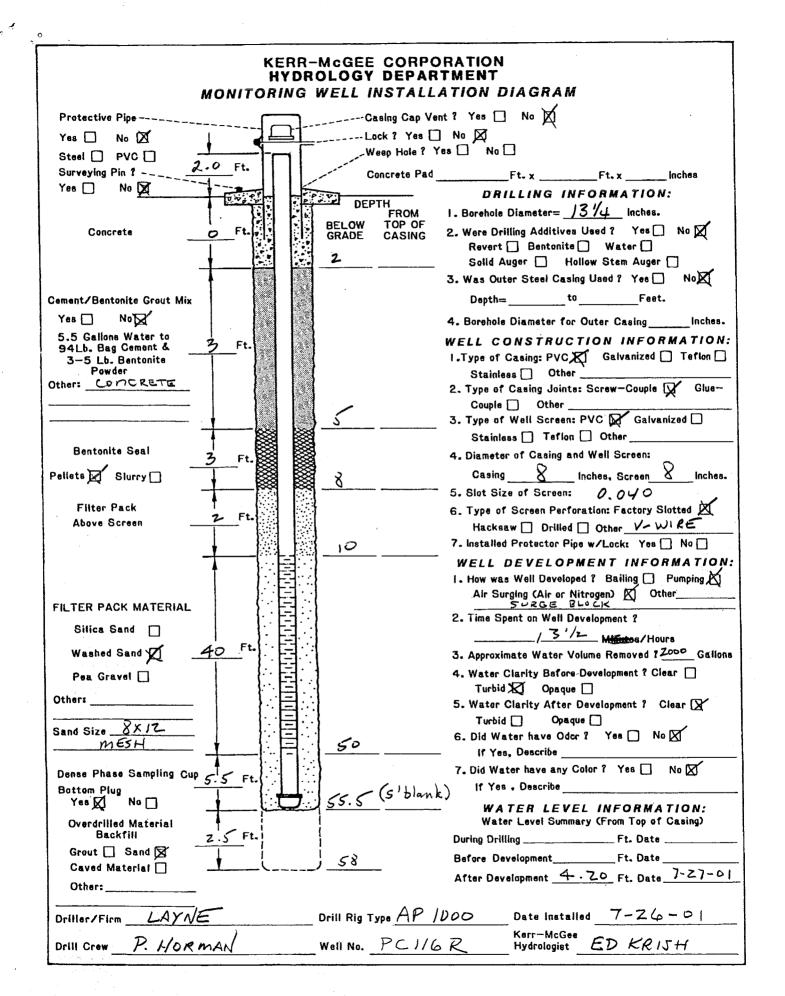
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SOIL BORING LOG KM-5655-B

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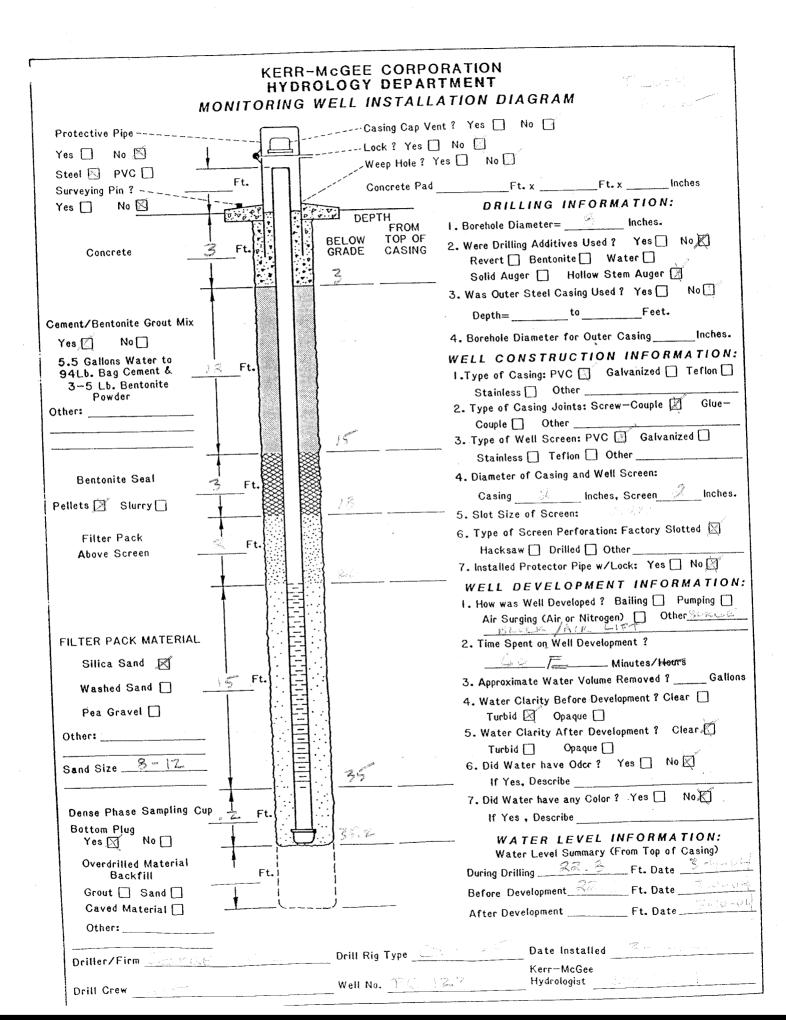
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KERR-McGEE CORPORATION KM SUBSIDIARY Hydrology Dept S&EA Division KMC LL				LC		HENT	ENST	al . A	$\overline{\mathbf{N}}$	V BORING PCILGR		
DEPTH	1	Kine			BLOWS		· · · · · · · · · · · · · · · · · · ·	SOIL SAI		1		
IN FEET	LITHOLOGIC DESCRIPTIC	N	GRAPHIC LOG	UNIFIED SOIL FIELD CLASS.	PER 6'	PID (ppm)		DEPT		REC.	REMARKS OR FIELD OBSERVATIONS	
45 <u>-</u>	<u>38-49</u> ' com. cobb 6"	1-5 -60	0.0010.00000000000000000000000000000000	GP								
49 _	- 49-58 CLAY & SIT Clay, W/rost tra Sm. gyp xtols.gree and blue green	ry ces é nish		CL							MC@49'	
-												
EXPLANATION	Water Table (24 Hour) Water Table (24 Hour) Water Table (Time of Borini PID Photoionization Detection (p NO. Identifies Sample by Numbry YPE Sample Collection Method SPLIT- BARREL AUGER THIN- WALLED CONTINUOUS SAMPLER DEPTH Depth Top ond Bottom of S REC. Actual Length of Recovered	ppm) er 5 R ample	OCK ORE ECOVE	RY		GRAPHIC I CLAY SILT SAND GRAVEL SILTY CLAYEY SILT			DRIL DRIL LOG	GED BY Ed	- DI Z of Z USSION NE	



SOIL BORING LOG KM-5655-A

KE	RR-McGEE CORPORATION	RY			LOCATION			·	BORING NUMBER PC 123			
Hydro	logy Dept. Engineering Services	KMC		Cor		HEN	DER			T	<u> </u>	
DEPTH IN FEET	LITHOLOGIC DESCRIPTI	ON	GRAPHIC LOG	UNIFIED SOIL FIELD CLASS.	BLOWS PER FOOT		NO.	SO TYPE	DEPTH	REC.	REMARKS OR FIELD OBSERVATIONS	
3 - - - - - - - - - - - - - - - - - - -	E- PANALANE ES 1-3 CARAVALASSA 65% INT. PEDIDIN 10% W/25% VD-V And 10% STH. 8-12 SAND, grave 70% W-VC, SA-SR 30% W-VC, SA-SR 30% W-VC, SA-SR 30% W-VC, SA-SR 30% W-VC, SA-SR 30% W-VC, SA-SR 12-ZZ SRAVEL OTO 70% VOIC PR SR, to 1/2-1" W/2 VC, SA-SR SL. 10% SA-SR	ravelly support ravelly ravelly ravelly ravelly ravelly ravelly ravelly ravelly ravelly		SW SW SW SW	~						DAMP @ 171	
	▼ Water Table (24 Hour) ▼ Water Table (Time of Bor Photoionization Detection NO. Identifies Sample by Nur Sample Collection Metho ▼ Split Split AUGER	(ppm) nber	ROCK			GRAPHIC CLAY Silt SAND		DE FIL ORG SA CL	BRIS L HLY SANIC (PEAT) NDY AY	DATE DRILLE 3-9- DRILLING ME DRILLED BY WD LOGGED BY	THOD THOD THIS Ages	
EXPLAN	DEPTH Depth Top and Bottom C REC. Actual Length of Recover	of Sample	NO RECOV			GRAVEL SILTY CLAY CLAYEY SILT		CL SA 	AYEY .ND		R GRID COORDINATES	



30 .9 (silt and clay) desc 30 .9 .9 30 .9 .9 30 .9 .9 30 .9 .9 30 .9 .9 31 .9 .9 32 .9 .9 33 .9 .9 34 .9 .9 35 .9 .9 35 .9 .9 36 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9 .9 37 .9	Elevation: 1615.08 FT St Mit Boring Diameter: 8 In. Sc Date/Time Started: 12/18/2008 11:30 Dete Date/Time Finished: 12/18/2007 15:00 W CATION, color, description of fine grained matription of coarse grained material (sand and pr mineralogical features, density or stiffness,	Well Diagram
inates: 26728191.37 N 829517.89 E Method: Sonic with continuous coring e Type(s): Split Spoon and Core Logged By:E. Krish Logged By:E. Krish vantez Backfill: NA 0 <	Elevation: 1615.08 FT St Mit Boring Diameter: 8 In. Sc Date/Time Started: 12/18/2008 11:30 Dete Date/Time Finished: 12/18/2007 15:00 W CATION, color, description of fine grained matription of coarse grained material (sand and primeralogical features, density or stiffness, odors or staining. AND, light brown (5YR 5/4), 10% silt, 35% fine grained regimer from 6-9", 55% very fine to very coarse grained regimer for the series of the series	ained rained aine ained aine ained aine aine aine aine aine aine aine aine
Method: Sonic with continuous coring e Type(s): Split Spoon and Core Logged By:E. Krish vantez Backfill: NA 0 0 </td <td>Mit Boring Diameter: 8 In. Sc Date/Time Started: 12/18/2008 11:30 De Date/Time Finished: 12/18/2007 15:00 With CATION, color, description of fine grained main ription of coarse grained material (sand and primeralogical features, density or stiffness, poors or staining. AND, light brown (5YR 5/4), 10% silt, 35% fine grained region of 1-3" from 6-9", 55% very fine to very coarse grained to very coarse grained region of the started s</td> <td>ained rained aine ained aine ained aine aine aine aine aine aine aine aine</td>	Mit Boring Diameter: 8 In. Sc Date/Time Started: 12/18/2008 11:30 De Date/Time Finished: 12/18/2007 15:00 With CATION, color, description of fine grained main ription of coarse grained material (sand and primeralogical features, density or stiffness, poors or staining. AND, light brown (5YR 5/4), 10% silt, 35% fine grained region of 1-3" from 6-9", 55% very fine to very coarse grained to very coarse grained region of the started s	ained rained aine ained aine ained aine aine aine aine aine aine aine aine
e Type(s): Split Spoon and Core Logged By:E. Krish vantez Backfill: NA 0 0 0	Boring Diameter: 8 In. Sc Date/Time Started: 12/18/2008 11:30 De Date/Time Finished: 12/18/2007 15:00 W CATION, color, description of fine grained matription of coarse grained material (sand and or mineralogical features, density or stiffness, odors or staining. AND, light brown (5YR 5/4), 10% silt, 35% fine grained region 1-3" from 6-9", 55% very fine to very coarse grained to very coarse grained regional coarse grained coarse grained regional coarse grained coarse grained regional coarse grained coa	ained rained rained cai
vantez Backfill: NA 0 0 <	Date/Time Finished: 12/18/2007 15:00 W CATION, color, description of fine grained mar ription of coarse grained material (sand and or mineralogical features, density or stiffness, odors or staining. AND, light brown (5YR 5/4), 10% silt, 35% fine gra or 1-3" from 6-9", 55% very fine to very coarse gr	ained rained ained ained ained ained ained Cement (94%) and Bentonite (6%) Slurry
MATERIAL IDENTIFI (silt and clay) desc gravel), structural of moisture content, of M	CATION, color, description of fine grained mai ription of coarse grained material (sand and or mineralogical features, density or stiffness, odors or staining. AND, light brown (5YR 5/4), 10% silt, 35% fine gra or 1-3" from 6-9", 55% very fine to very coarse gr	terial Well Diagram
ALLUVIUM: GRAVELLY S M gravel to 3/4" with mir	ription of coarse grained material (sand and or mineralogical features, density or stiffness, odors or staining. AND, light brown (5YR 5/4), 10% silt, 35% fine gra or 1-3" from 6-9", 55% very fine to very coarse gr	ained rained PVC Riser Cement (94%) and Bentonite (6%) Slurry
M [· · · []] gravel to 3/4" with mir	10r 1-3" from 6-9" , 55% very fine to very coarse gr	-2" Sch. 40 PVC Riser -Cement (94%) and Bentonite (6%) Slurry
		—Bentonite Seal
M O O O O O O O O O O O O O O O O O O O	wn (5YR 6/4), 10% silt, 40% very fine to very coars subrouned sand, 50% fine grained gravel to 3/4"	with
	C .	
	-	
M SILTY GRAVELLY SAND, 35% fine grained ang	dark yellowish brown (10YR 4/2), locally up to 25% ular to subrounded volcanic pea gravel, up to 40%	
M ∘ () ¶ ↓ to coarse grained sar	id in matrix, 10-20% silt.	y fine
P	SANDY GRAVEL, at 17.5 fr SANDY GRAVEL, caliche z SANDY GRAVEL, caliche z SANDY GRAVEL, caliche z SANDY GRAVEL, caliche z SANDY GRAVEL, groundw SILTY GRAVELLY SAND, 35% fine grained ang fine to very coarse gra and clean sand.	SANDY GRAVEL, at 17.5 feet bgs cobbles to 6". SANDY GRAVEL, caliche zone from 19-19.5 feet bgs. SANDY GRAVEL, caliche zone from 22.5-23 feet bgs. SANDY GRAVEL, caliche zone from 22.5-23 feet bgs. SANDY GRAVEL, groundwater encountered at 32 feet bgs. SILTY GRAVELLY SAND, dark yellowish brown (10YR 4/2), locally up to 25% 35% fine grained angular to subrounded volcanic pea gravel, up to 40% fine to very coarse grained subangular to subrounded sand, alternating and clean sand. SANDY SILTY GRAVEL, brownish gray, very hard calichification, 20-30% very to coarse grained sand in matrix, 10-20% silt. SANDY SILTY GRAVEL, from 32.5-33 feet bgs very silt-40%.

WELL CONSTRUCTION TRONOX TRONOX CAPTURE WP.GPJ ENSR CA.GDT 4/25/08

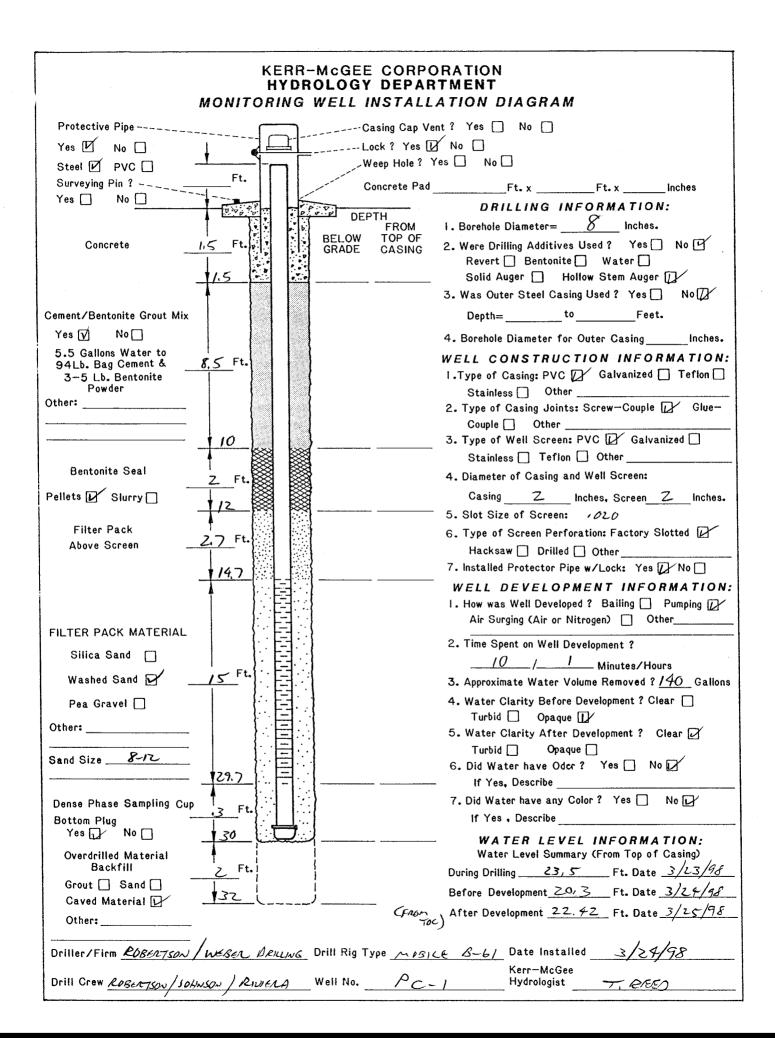
EN	NSR	AEC	COM			oject N		Tronox LLC 04020-023-160			Well No. PC-136		
	EN: 220 Aven amarillo,	ida Aca CA 930			Со	ordina	tes:	26728191.37 N 829517.89 E Sonic with continuous coring		Sheet: 2 Monitoring V	of 2 Vell Installed: Yes		
	(805)38	8-3775			Sar	mple 1	Type(s):	Split Spoon and Core	Boring Diameter: 8 In.	Screened Interval: 17.7-37.7 ft.			
Weathe	r: 1	A						Logged By: E. Krish	Date/Time Started: 12/18/2008 11:30	Depth of Bo	<i>ring:</i> 38 ft.		
Drilling (Contracto	or: Bo	art Lor	ngyea	r / D. (Cerva	ntez	Backfill: NA	Date/Time Finished: 12/18/2007 15:00	Water Level	: Not Encountered		
DEPTH (ft)	Sample ID	Sample Depth (ft)	Blows per 6"	Recovery (ft)	Headspace (ppm)	nscs	Graphic Log	(silt and clay) des gravel), structural moisture content,	,	and ness,	Well Diagram		
						GP- GM CL		to coarse grained sa	prownish gray, very hard calichification, 20-30 ind in matrix, 10-20% silt. <i>(continued)</i> ION: CLAY, light greenish gray (10Y 7/1).	% very fine			
No	tes:												

Client: Tronox LLC ENSR AECOM Project Number: 04020-023-160									Well No	. PC-137					
LI	IJI	TILL	-OIVI			-		ocation: East Side of Athens	Road Well Field, He	nderson, NV	1				
	ENS	SR				ordina	-	26728198.98 N 829517.57 E			Sheet: 1	of 2			
12 Ca	220 Aveni amarillo, (ida Aca CA 930	so 12		Dri	lling M	lethod:	Sonic with continuous coring	Sonic with continuous coring						
	(805)388	8-3775			Sa	mple 1	Type(s): S	Split Spoon and Core	Boring Diameter:	8 In.	Screened	Interval: 59.	7-69.7 ft.		
Weather: NA						Logged By: E. Krish		12/17/2007 14:15	Depth of B	oring: 70 ft.					
Drilling C	Contracto	1	art Loi	ngyea		Cerva	ntez	Backfill: NA	Date/Time Finished	: 12/17/2007 17:30	Water Leve	e <i>l:</i> 28 ft.			
DEPTH (ft)	Sample ID	Sample Depth (ft)	Blows per 6"	Recovery (ft)	Headspace (ppm)	nscs	Graphic Log	(silt and clay) des	cription of coarse gi or mineralogical fe	scription of fine grained rained material (sand a atures, density or stiffn	nd	w	'ell Diagram		
						SP- SM		ALLUVIUM: GRAVELLY S very coarse grained,	subangular to subro	unded sand, 20% fine gra	ained		-Flush Mount		
								volcanic pea gravel, moderately soft calc	subangular to subrou	inded to 3/4" with minor 1	1-2",				
								moderately soft calc	areous grain coatings	5.			2" Sch. 40 PVC Riser		
 						GP- GM		SANDY GRAVEL, light bro grained subangular t pea gravel to 1/4", n -groundwater encoutered a	o subrounded sand, noderate calcareous	60% fine, angular to subr	coarse rounded,				
						SP	6767	GRAVELLY SAND, moder	ate brown (5YR 4/2)	5% silt 15% fine grained	d angular to				
								subrounded , volcani subangular to subrou	c pea gravel to 3/8",	80% very fine to very coa	irse grained,		Cement (94%) and Bentonite		
													(6%) Slurry		
30													ł		
													\$		
													}		
						GM	[KH]	SANDY SILTY GRAVEL, v very coarse grained	subangular to subrou	inded sand, 50% fine gra			}		
							Potol	angular to subangula	r pea gravel to 3/8" v	with minor 1".			\$		
							eXP	-hard calichified zone from	34-36 feet bgs.				ł		

ENSR AECOM Project Number: 04020-023-160 Site Description/Location: East Side of Athens Road Well Field, Henderson, NV									Well No. PC-137					
					Site	Desc	cription/Lo	ocation: East Side of Athens	Road Well Field, Henderson, NV					
	ENS				Coc	ordina	tes: 2	26728198.98 N 829517.57 E	Sheet: 2	of 2				
12 Ca	20 Aveni amarillo, (CA 930	so 12		Drill	ling M	lethod:	Sonic with continuous coring	Monitoring	Monitoring Well Installed: Yes				
	(805)388	3-3775			San	nple 7	Type(s):	Split Spoon and Core	Boring Diameter: 8 In.	Screened	Interval: 59.7-69.7 ft.			
Veather.	: N	IA						Logged By: E. Krish	Logged By:E. Krish Date/Time Started: 12/17/2007 14:15 D					
Drilling C	ontracto	r: Boa	art Lor	ngyea	r / D. C	Cerva	ntez	Backfill: NA	Date/Time Finished: 12/17/2007 17:30	Water Lev	<i>el:</i> 28 ft.			
DEPTH (ft)	Sample ID	Sample Depth (ft)	Blows per 6"	Recovery (ft)	Headspace (ppm)	nscs	Graphic Log	(silt and clay) des	ICATION, color, description of fine grair cription of coarse grained material (sand or mineralogical features, density or sti odors or staining.	d and	Well Diagram			
						GM	exp							
 						CL- ML CL- ML		CLAYEY SILT, yellov plastic fines with up t -light greenish gray (5GY & -yellowish gray (5Y 7/2) fro	om 40 to 49 feet bgs. een (5Y 6/2) to dark gray (5Y 9/1) from 49 to Y, medium gray (N5) and light gray (N7) 25	ninately low	-Bentonite Seal			
_ 55						ML			v (5Y 6/4) , 20% very fine grained sand.		Sand Pack			
						ML		SANDY SILT, pale olive (1			#2-12			
<u>60</u>		\times		1.5		SM CL			e gray (5B 5/1), 30% silt, 70% very fine grai Y SILT, greenish gray (5G 6/1), disseminate					
						SM		SILTY AND CLAYEY SAN	D, greenish gray (5G 6/1), 40% silty clay, 6 ninated very fine grained marcasite.	0% very fine	귀: :[클::]			
<u>65</u>						CL		CLAY WITH GYPSUM CR gypsum crystals 3/8	RYSTALS, dark yellowish brown (10YR 4/2)		Well Screen (2" Sch. 40 PVC 0.01" Slot)			
70 Not	tes:							Total Depth = 70 feet. Boring Terminated Target depth achieved						

	KERR-McGEE CORPORATION KM SUBSIDIARY Hydrology Dept S&EA Division KmcLLC						LOCATION	(a. i			BORING NUMBER PC-1		
DEP	1		Noic	4.3	UNIFIED	BLOWS		$\frac{c_{sv}}{c_{sv}}$		AMPLE	E		
IN FE	1	LITHOLOGIC DESCRIPTIC	м	GRAPHIC LOG	SOIL FIELD CLASS.	PER 6'	PID (ppm)	NO.	ш	EPTH	REC.	REMARKS OR FIELD OBSERVATIONS	
		FILL: SAND /GRAVEL	N	51	-								
		IMPOUNDMENT BERM		M			<u> </u>					-	
				V.			_					-	
5							<u> </u>						
		SAND / SILTY SAND; C BROWN; GRAVEL COMM		10									
	-	WELL-GRADED; DRY		10,								_	
10				, o									
	_			Y	Sm-							—	
		SAND AS ABOVE	~	0									
	4				GM							_	
15				. 4 . j								_	
		CALICHE ZONE C 17-19		1								_	
	-			000	2							_	
20		SAND AS ABOVE, BECOM	~6	91.	-		<u> </u>						
	_	MUIST GRAVEL ZONE C 22'		0								_	
20				ee d	_7.		Ē						
23	.5 <u>X</u> -			Γ. Ö		23.7	†					_	
				F O			<u> </u>					GROUNDWATER -	
	_			10			<u> </u>					SAMPLE COLLECTES -	
				0	,		F					C 28'	
30				C.J.									
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SOIL BORING LOG KM-5655-B



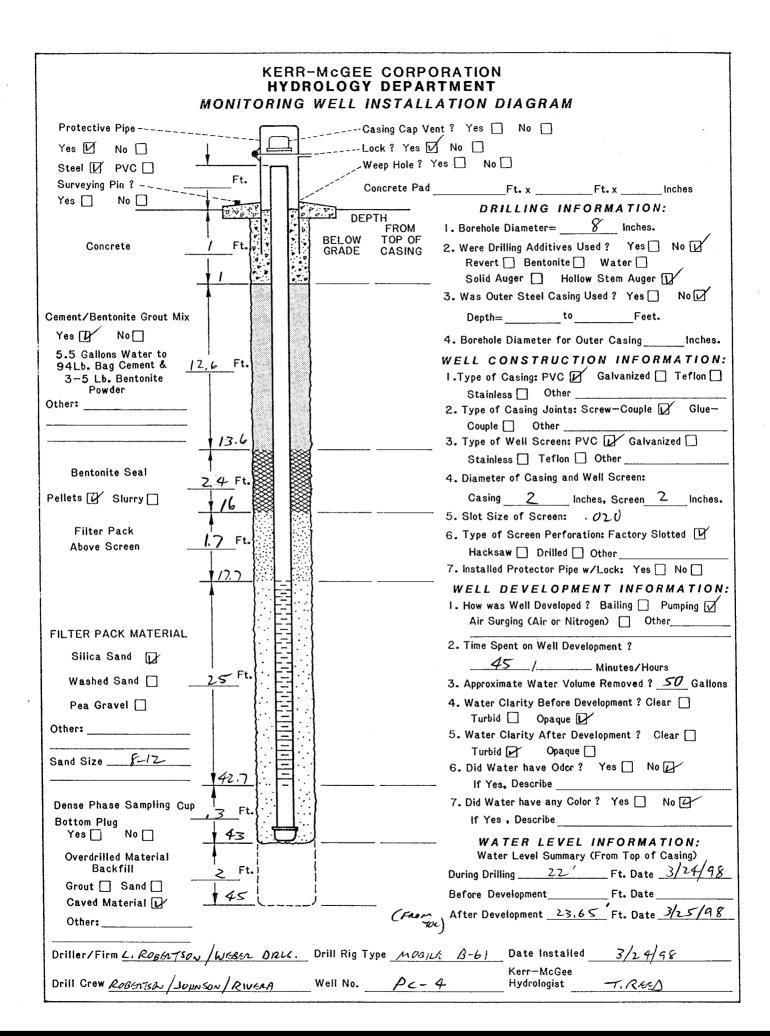
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SOIL BORING LOG KM-5655-B

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SOIL BORING LOG KM-5655-B

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Appendix B

Research Laboratory Bench-Scale Testing Protocols

TREATABILITY STUDIES FOR PERCHLORATE FROM AQUIFER MATERIAL AT THE NEVADA ENVIRONMENTAL TRUST SITE

John H Pardue PhD, PE and W. Andrew Jackson, PhD, PE

Creation of a permeable reactive barrier (PRB) is one strategy to reduce perchlorate to nontoxic end products in contaminated aquifers. Kinetic information on perchlorate reduction and the identity of suitable electon donors is required to effectively design PRBs for this purpose. The treatability studies proposed below are designed to identify suitable electron donors that will drive perchlorate reduction without seriously impacting the permeability of the formation or causing unaceptable downgradient water quality impacts. The site of interest is the Nevada Envrionmental Response Trust (NERT) site in Henderson, NV. Based on previous microcosm studies, perchlorate reduction is electron donor limited in the Las Vegas wash and in the contaminaed groudwater (Battista et al., 2003). Reduction will not occur in the absence of a supplemental carbon source. Required dosage is unknown and depends on the background demand from other electon acceptors and the demand from perchlorate reducers. The goal of these treatability studies is to identify the identity and dose of a suitable carbon source.

1.1 Objectives

The overall objective of these bench-scale studies is to ensure success for a pilot PRB. The specific objectives of the proposed bench-scale treatability studies are:

- 1. Identification of suitable electron donors for perchlorate reduction
- 2. Measurement of perchlroate reduction kinetics in NERT aquifer material.
- 3. Establish kinetic and hydraulic parameters required to design a PRB pilotTasks

1.2 Tasks

Task 1. Identification of suitable organic donors

Soluble, slow-release and solid electron donors will be tested to establish candidate amendments for perchlorate reduction in the PRB pilot. Example soluble donors may inlcude acetate, lactate or mixed donors (e.g., yeast extract) (Coates and Jackson, 2009). Proprietary slow-release donors will also be tested. These will be contrasted with a mixture of peat and sand to mimic constuction of a PRB out of a solid electron donor instead of amendment of the existing aquifer material. A total of 8-10 donors will be evaluated. Final selection of the amendments will be made jointly with ENVIRON. To establish effectiveness, serum bottle testing will be conducted on mixtures of site aquifer material, site groundwater and different concentrations of candidate donors. Testing will be conducted using methods described in the attached SOP. Briefly, materials will be assembled in a glove box in 160 mL serum bottles sealed with Teflon-lined septa and crimp caps (Tan et al., 2004 and Jackson et al., 2004). Bottles will be repetitvely sampled over time to establish the kinetics of perchlorate reduction. In addition to perchlorate, concentrations of relavent redox pairs will be measured as the

changes in the aquifer material/groundwater systems progress. These will include oxygen, nitrate/nitrite, ferric/ferrous iron, sulfate/sulfide and methane. Studies will be run for 6-8 weeks or until the perchlorate is reduced by 80-90%. Successful electron donors will be evaluated based on kinetics of perchlorate reduction and mitigation of lag time due to presence of oxygen and nitrate. Cost and implementability will be additional strong considerations for candidate donors for further evaluation in 1-D columns.

Task 2. Assessment of perchlorate reduction kinetics in 1-D columns

Coumn studies will be used to test the effectivess of donors in a flow-through mode. Successful donors will be those that reduce perchlorate but also maintain the hysraulic properties of the formation (minimize biofouling). A schematic diagram of the 1-D column system is shown in Figure 1. Column experiments will be performed in three, 5 ft long, 2 inch diameter columns with 5 equispaced sampling ports located along their lengths. The columns will be packed with aquifer material from the NERT site. A 5 cm layer of fine gravel will placed at the bottom for even distribution of flow through the column. Glass wool will be inserted in the inner side of sampling ports to avoid dead zones and clogging of sampling ports. Immediately after establishment of the columns, the hydraulic conductivity of the test columns will be measured using the falling head method and compared to existing site data.

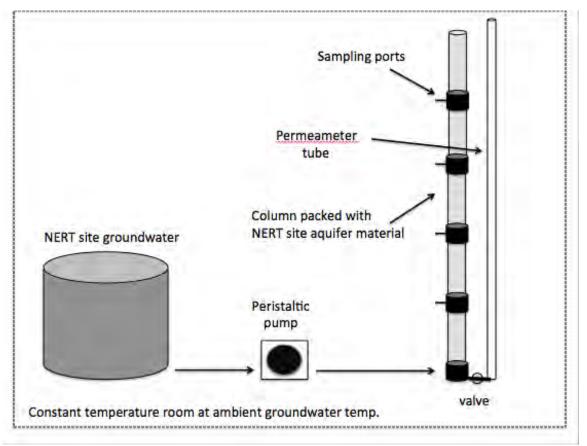


Figure 1. Column set-up

Contaminated groundwater, shipped from the site, will be introduced through 2 mm stainless steel tubing in upflox mode. A peristaltic pump (Cole Parmer Masterflex) with Viton tubing will used to convey water through the column at groudnwater velocities representative of site conditions. The experiment will be set-up in a constant temperature room so that site groundwater and the test columns will be maintained at the ambient site temperature.

The influent concentrations will be monitored three times a week to track changes in perchlorate concentration. Influent samples for all column experiments will be collected at the sampling ports on the delivery side of the pump. Samples were collected with a 5 mL prerinsed airtight glass syringe fitted with luer-lock and injected into 2 mL glass vials. Sampling was performed after every three-four days for determination of perchlroate concentration, nitrate/nitrate concentrations and conductivity. On a weekly basis, additonal redox indicators will be measured including O2, nitrite, nitrate, ferrous iron, ferric iron, sulfate and sulfide, and methane. Redox characteristics of each sampled zone would be determined from these multiple lines of evidence from the water chemistry testing. Additional samples will be removed for metals analysis at an external certified laboratory acceptable to ENVIRON. Column studies will be run for 12 weeks, subject to extension if additional information is desired. Following the termination of the studies, the falling head permeameter study will be repeated and the hydraluc conductivity measured again. Declines in conductivity over the 12 weeks may be evidence of biofouling. If conductivity declines significantly (>5-10x), column materials will be removed and total carbon measured on the aquifer material to determine the amount of biomass accumulated along the flowpath.

Task 3. Establishing kinetic and hydraulic parameters

Column data for removal of perchlorate can be assessed using 1-D reactive-transport models:

$$\frac{\partial C}{\partial t} = -\frac{u_x}{R} \frac{\partial C}{\partial x} + \frac{D_x}{R} \frac{\partial^2 C}{\partial x^2} - \frac{k}{R}C$$

Because of the uncertainty in the scale-dependent dispersion term, D_x (the dispersion term is very small over the short depth of the columns), a simpler exponential equation can also be used to assess kinetics for pechlorate treatment.

$$C = C_o e^{-kRx/v}$$

where $C [M/L^3]$ is the concentration of the pollutant at a vertical distance, x [L], $C_o [M/L^3]$ is the initial concentration, $k [T^{-1}]$ is a lumped temporal degradation rate constant, R is the retardation coefficient and v [L/T] is the seepage velocity. The equation captures several important mechanisms including equilibrium partitioning, advection and first-order reduction of perchlorate. Partitioning is expected to be negligible for perchlorate (e.g., R=1). Biodegradation rate constants will be determined by fitting the equation to contaminant profiles measured in Task 2 using CXTFIT, a curve fitting program used for 1-D column

studies (Toride et al., 1995) or using non-linear regression for the simpler exponential equation.

1.2.1 Analytical Procedures

Major anions (Cl⁻, NO₃⁻, and SO₄²⁻) will analyzed by ion chromatography following EPA Method 300.0. ClO₄⁻ concentrations will be separately measured by sequential ion chromatography-mass spectroscopy-mass spectroscopy (IC-MS/MS). ClO₄⁻ will quantified using a Dionex LC 20 ion chromatography system consisting of GP50 pump, CD25 conductivity detector, AS40 automated sampler and Dionex IonPac AS16 (250 X 2 mm) analytical column. The IC system is coupled with an Applied Biosystems – MDS SCIEX API 2000TM triple quadrupole mass spectrometer equipped with a Turbo-IonSprayTM source. A hydroxide (NaOH) eluent at 0.3 mL min⁻¹ is followed by 90% acetonitrile (0.3 mL min⁻¹) as a post-column solvent. To overcome matrix effects, all samples were spiked with Cl¹⁸O₃ or Cl¹⁸O₄ internal standards. Redox paramaters will be measured using standard methods O₂ (microelectrode), nitrite, nitrate, ferrous, ferric iron, sulfate, sulfide (ion chromatograph), methane in porewater (GC-FID), SOPs of each of these measurements are available upon request.

QA/QC

Full details of QA/QC procedures are available in the SOPs. Briefly, the QC program consists of blanks, calibration checks, matrix spikes and matrix spike duplicates. Our QA/QC for these parameters has been approved by a number of agencies including the US Army, Florida DEQ and others. Split samples will be provided for analysis at external laboratories at ENVIRON's request.

References:

Batista, Jacimaria R., Amy, Penny S., Chen, Yi-Tung, Papelis, Lambis, Unz, Richard. 2003. The Fate and Transport of Perchlorate in a Contaminated Site in the Las Vegas Valley. Part A: Investigation of the Influence of Biological Degradation and Sorption on the Fate of Perchlorate. Part B: Modeling of the Transport of Perchlorate in the Las Vegas Wash. Final Project Report EPA Grant Number: R827622E03

J. Coates and W.A. Jackson. 2009. Development of *In Situ* Bioremediation Technologies for Perchlorate in <u>In Situ Bioremediation of Perchlorate in Groundwater</u>. Hans F. Stroo and Herb Ward, (Eds.) 2009, XLVI, PP. 29-53.

Jackson, W. Andrew, M. Jeon, J. H. Pardue, and T. Anderson. 2004. Enhanced Natural Attenuation of Perchlorate in Soils Using Electrokinetic Injection. <u>Bioremediation Journal</u>. 8(1-2):65-79.

Tan, K., T.A. Anderson and W.A. Jackson. 2004. Degradation Kinetics of Perchlorate in Sediments and Soils. <u>Water, Air, and Soil Pollution</u>. 151:245-259.

Toride, N., F. J. Leij, and M. Th. van Genuchten. 1995. The CXTFIT Code for Estimating Transport Parameters from Laboratory or Field Tracer Experiments, Version 2.0. Research Report No. 137, U.S. Salinity Laboratory, USDA, ARS, Riverside, California.

Appendix C

Standard Operating Procedures

Standard Operating Procedure C-1:

Soil Sampling with Direct-Push or Hollow-Stem Auger Samplers

Soil Sampling with Direct-Push or Hollow-Stem Auger Samplers

April 22, 2014

ENVIRON International Corporation

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1. Purpose

This standard operating procedure (SOP) is applicable to the collection of representative soil samples using a direct-push or hollow-stem auger sampling technique. The methodologies discussed in this SOP are generic in nature and may be modified in whole or part to meet the handling and analytical requirements of the contaminants of concern, as well as the constraints presented by site conditions and equipment limitations. Modifications of sampling methodologies will be documented in the appropriate field logbook and discussed in reports summarizing field activities and analytical results. For the purposes of this procedure, soils are those mineral and organic materials not submerged in water for an extended period of time sufficient to support aquatic life.

2. Sample Collection

The primary means for the collection of subsurface soil samples will be a direct-push technique using a Geoprobe[®] or equivalent driver. Direct-push soil samples will be obtained using a closed-piston soil sampler with a liner (or equivalent sampling system). If needed, a hollow-stem auger sampler may be used to collect soil samples. The sampler will be operated in accordance with the manufacturer's recommended operating procedures for the type of equipment used.

2.1 Discrete Soil Sampling Procedures

Soil samples will be collected at predetermined intervals based on specific data needs. Each discrete sample will be described in the field notebook using the Unified Soil Classification System (USCS) as described below. Soil samples that will not become composite samples will be placed directly in the appropriate sample containers using a clean plastic or metal spatula, or by using a clean gloved hand.

Subsamples selected for laboratory analysis will be placed in appropriate sample containers provided by the analytical laboratory, labeled, placed in an iced cooler, and stored in accordance with chain-of-custody requirements specified in the Quality Assurance Project Plan (QAPP) until shipment to the laboratory (or laboratories) is arranged. Chain-of-custody records will be completed for all samples according to the methods described in the QAPP.

Discrete samples that will become aliquots of a composite sample will be covered or capped as soon as possible after collection if the compositing process is not completed immediately. Each sample container will be labeled and stored on ice pending the composite process.

2.2 Composite Soil Sampling Procedures

Composite samples will be prepared from the discrete samples following collection of the required number of discrete sample specified for the sampling area. Each discrete sample will be removed from the sample container and placed on a clean sheet of aluminum foil. After removing sticks, grass, stones, and other debris, each discrete sample will be separated into quarters – cores will be cut lengthwise into 4 equal portions, while disturbed samples will be homogenized and divided. Three of the four quarters of each sample will then be placed into

one of three individual foil pans. The fourth portion of the discrete sample will be placed in a plastic baggie, labeled, sealed, and stored separately for potential individual analysis.

The compositing process of quartering discrete samples will be repeated for successive discrete samples until each of the three pans contains one quarter of each discrete sample. The contents of each aluminum foil pan will then be thoroughly mixed either by hand or by using an electrical or mechanical mixer. Upon completion of the mixing process, the contents of each individual pan will then be combined into one clean pan and again thoroughly mixed, resulting in one homogeneous sample. The composite soil sample will then be placed in the appropriate sample containers, labeled, and placed on ice pending shipment to the laboratory.

2.3 VOC Sample Collection Procedures

Soil samples obtained for laboratory analysis of VOCs will be collected in compliance with SW-846 Method 5035. Each soil sample will be obtained directly from the sampling device (i.e., not homogenized) using an En Core[™] sampler or field preserved using Method 5035 compatible containers. A description of each sampling procedure is as follows:

EnCore Sampler

The EnCore[™] sampler is a single use, commercially available device constructed of an inert composite polymer. EnCore[™] uses a coring/storage chamber to collect either a 5-gram or 25-gram sample of cohesive soils. It has a press-on cap with a hermetically vapor tight seal and a locking arm mechanism. Three EnCore[™] samplers shall be filled at each sample location using the following procedures:

- Place the EnCore[™] sampler into the EnCore[™] T-Handle tool.
- Push the sampler into the soil sample until the small o-ring on the plunger of the EnCore[™] sampler is visible in the T-Handle viewing hole.
- Wipe off any excess soil from the coring body exterior using a clean paper towel.
- Place the cap on the end of the EnCore[™] sampler and twist to lock the cap into place.
- Remove the sampler from the T-Handle and lock the plunger by rotating extended plunger rod fully counterclockwise until the plunger wings rest firmly against the plunger tabs.
- Place the label on the sampler and place the sampling into a labeled EnCore[™] sampler bag and zip closed.
- Place the filled EnCore[™] samplers in a cooler with ice for overnight shipment to the laboratory using standard chain-of-custody procedures. The soil samples must be prepared for analysis or frozen within 48 hours of sample collection.

Field Preservation

The procedures for the field preservation method are as follows:

• Push a one-time use plastic sampling tool such as a Terra Core[™] sampler into the soil to be samples to collect an approximately 5-gram sample aliquot.

- Transfer the 5-gram aliquot to laboratory provided, pre-preserved, 40-milliliter vials containing a specific amount of methanol, sodium bisulfate, and/or organic-free water. The number of vials provided with each preservative will vary by the laboratory performing the analysis. One unpreserved container shall also be filled to allow for laboratory calculation of the sample dry weight.
- Label each sample and place in a cooler with ice for overnight shipment to the laboratory using standard chain-of-custody procedures.

3 Sample Description and Field Documentation

After samples for chemical and physical analysis have been prepared, a visual soil or lithologic description of each sample will be made according to the USCS, and will be recorded in a bound log notebook. Each sampling location will be photographed, and the approximate location will be placed on a site map and recorded in the field notebook.

Residual soil from the compositing process and stored individual discrete sample portions will be disposed in accordance with the Sampling and Analysis Plan.

4 Equipment Decontamination

Drilling and support equipment will not come in direct contact with the samples, so crosscontamination of samples is not a concern. However, this equipment will likely come in contact with impacted soil and must therefore be decontaminated prior to moving from one location to another.

The drilling equipment used for soil sampling and monitoring well installation will be cleaned with high-pressure/hot water washing equipment prior to initiating the field investigation. The same procedure will be applied to all drilling equipment between each boring location. The cleaning will occur at a decontamination pad constructed at a suitable location(s) at the site. Water used for cleaning will be obtained from a local potable water source. Equipment subject to these decontamination procedures includes, but is not limited to, the following:

- Direct-push or hollow-stem auger drill rig.
- Direct-push or hollow-stem auger sampler components.

In addition, downhole equipment that comes in direct contact with samples will be decontaminated between each sample interval. This procedure will include washing with a nonphosphate detergent and rinsing with clean potable water.

If required, a piece of sampling equipment that comes in direct contact with soil samples (e.g., split-barrel samplers) will be selected for collection of field equipment blanks. After the equipment has been cleaned, it will be rinsed with DI water. The rinse water will be collected and submitted for analysis of all constituents for which the normal samples collected with the equipment are being analyzed.

Field blanks will be collected at the frequency specified in the QAPP.

5 References

ENVIRON. 2014. Quality Assurance Project Plan, Nevada Environmental Response Trust, Henderson, Nevada. January 24.

United States Environmental Protection Agency. 1996. Method 5035: Closed-System Purgeand-Trap and Extraction for Volatile Organics in Soil and Waste Samples. December.

Standard Operating Procedure C-2:

Low-Flow Groundwater Sampling for Chemical Analysis

Low-Flow Groundwater Sampling for Chemical Analysis

April 22, 2014

ENVIRON International Corporation

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1 Purpose and Scope

This standard operating procedure (SOP) describes the procedures to be followed by a Field Geologist/Engineer while collecting groundwater samples using low-flow purging and sampling procedures. The low-flow methodology may alternatively be referred to by names such as "micropurging", "low-stress purging", low-impact purging, or "minimal drawdown purging." This SOP should be used primarily for collection of groundwater samples from permanent wells that have been designed, constructed, and developed for the purpose of monitoring groundwater. The groundwater samples that are collected using this SOP are acceptable for the analysis of environmental contaminants including, but not limited to: volatile organic compounds (VOCs), semi-volatile organic compounds (SVOCs), pesticides and herbicides, polychlorinated biphenyls (PCBs), petroleum hydrocarbons, metals, and other inorganic compounds.

The procedures presented herein are intended to be of general use and may be supplemented by a Work Plan, Sampling and Analysis Plan, Quality Assurance Project Plan, and/or a Health and Safety Plan. Some of these procedures may not be required depending on the specific scope of work being conducted. As the work progresses, and if warranted, appropriate revisions may be made by the Project Manager. Procedures in this protocol may be superseded by applicable regulatory requirements.

2 General Requirements

All personnel performing on-site operations with the potential for exposure to hazardous substances or health hazards are required to be 40-hour trained in accordance with Code of Federal Regulations (CFR) 1910.120 and will meet the personnel training requirements in accordance with 29 CFR 1910.120(e).

The laboratory must be certified by the appropriate regulating agency for the analyses to be performed. If drilling is required as part of the scope of work, permits will be acquired from the appropriate agency, and an underground utility check will be performed before drilling begins. An underground utility check will, at a minimum, consist of contracting with a local utility alert service, if available. Under certain circumstances, including at sites with deeply buried, unknown, or multiple underground utilities, as well as at high risk sites such as oil refineries and heavy industrial facilities, manual utility clearance using hand auger or air knife methods should also be performed.

The activities described in this SOP require the implementation of a site-specific Health and Safety Plan to inform personnel of the hazards associated with this work and to describe the methods that will be employed to mitigate those hazards. The Health and Safety Plan must be prepared and approved by the Project Manager and the local Health and Safety Coordinator prior to initiating field work. A Health and Safety Meeting must be held at the start of each day to reassess any potential hazards associated with that day's field work.

3 Methods

This SOP has been prepared in accordance with the United States Environmental Protection Agency (USEPA) Standard Operating Procedure for Low-Stress (Low Flow)/Minimal Drawdown

Ground-Water Sample Collection, dated 2002. This guidance document is included as Attachment 3 of the Ground-Water Sampling Guidelines for Superfund and RCRA Project Managers, which may be found via the following internet link:

http://www.epa.gov/swertio1/tsp/download/gw sampling guide.pdf

This methodology described herein is also consistent with the California Environmental Agency's (Cal-EPA), Representative Sampling of Groundwater for Hazardous Substances, Guidance Manual for Ground Water Investigations, dated June 2005. This document may be found via the following internet link:

http://www.dtsc.ca.gov/SiteCleanup/upload/SMP Representative Sampling GroundWater.pdf

Unlike traditional purging methods, low-flow purging and sampling does not require the removal of an arbitrary volume of water from a well prior to sampling. Instead, low-flow purging and sampling relies on careful monitoring of water quality indicator parameters to determine when a representative groundwater sample can be collected. The low-flow methodology minimizes the effects on groundwater chemistry caused by the purging process by minimizing drawdown, reducing the amount of water removed from the well, and reducing the amount of turbidity in groundwater samples.

4 Equipment and Materials

A non-exhaustive summary of common supplies and equipment is presented below:

- Health and Safety Plan
- Site information (maps, contact numbers, previous field logs, etc.)
- Electronic water level indicator (Solinst or similar)
- Photoionization Detector (PID) of Flame ionization detector (FID) if VOCs are suspected
- Adjustable-rate sampling pump capable of rates <0.5 liters per minute (bladder pump preferred, e.g., QED Sample Pro)
- Bladders for sample pump
- Sample tubing (Teflon® or Teflon®-lined tubing preferred for sampling organic compounds)
- Multi-parameter meter (e.g. YSI 556 Multi-Parameter Meter) with flow through cell capable of measuring (at a minimum) temperature, pH, specific electrical conductance (SEC), dissolved oxygen (DO), and oxidation-reduction potential (ORP)
- Turbidity meter
- In-line filters (if required, e.g. for dissolved metals)
- Certified-clean sample containers and preservation supplies, sample labels, Ziploc[™] bags

- Cooler with ice
- Decontamination supplies (e.g. phosphate-free detergent, distilled water)
- Tool kit with appropriate tools (socket wrench set, pry bar, Dolphin locks/keys)
- Drum(s) to collect purged water and decontamination water
- Drum labels
- Person Protective Equipment (PPE), typically PPE will consist of:
 - Long-sleeved shirt and long pants
 - Steel-toed boots
 - Hardhat
 - Nitrile gloves
 - Safety glasses with side shields
 - Other as required by Health and Safety Plan
- Field Forms (If the project requires it, a project-specific Field Logbook may substitute for any of the following with the exception of the Chain of Custody)
 - Field Investigation Daily Log
 - Water Level Measurement Log
 - Low-Flow Purging and Sampling Log
 - Equipment Calibration Log
 - Chain-of-Custody

5 Procedures

The following sections discuss the procedures to follow during low-flow purging and sampling monitoring wells with dedicated or non-dedicated equipment (e.g., bladder pumps with adjustable rate controls). Where applicable and when possible, the purging and sampling techniques should remain consistent from one sampling event to the next.

5.1 **Pre-Sampling Activities**

- 1. Sampling should begin at the monitoring well with the least contamination, generally upgradient or farthest from the site or suspected source. Then proceeding systematically to the monitoring wells with the higher expected groundwater concentrations.
- 2. All measuring devices and monitoring equipment should be calibrated according to manufacturer's recommendations. Water quality meters must be calibrated daily before use. Equipment calibration details should be recorded in the *Equipment Calibration Log*.
- 3. Unlock well and/or remove well cap. Record any damage or evidence of pressure (positive or negative) in the well in the Water Level Measurement Log. Monitor the headspace at the top of the well for VOCs with a PID or FID and record findings. If VOCs are present, monitor worker breathing zones during purging and sampling in accordance with the site

Health and Safety Plan.

- 4. Prior to sampling, the depth-to-water in all wells must be measured to obtain the current static water level. Water levels should be measured to the nearest 0.01 feet relative to a reference measuring point on the Top of Casing (TOC) which must be surveyed relative to ground elevation. If there is no marked reference point on the TOC, measure from the North side of the casing. Record depth to groundwater information in the *Water Level Measurement Log*. The same water level measuring device should be used for all wells, if possible, and must be decontaminated between each well.
- 5. Use existing site information for total depth (TD) of monitoring well and use the information from depth to water to calculate the volume of water in the monitoring well. The TD of wells to be sampled should not be tagged prior to sampling to avoid disturbing sediments at the bottom of the well. If possible, have this information prior to the day of sampling. The TD of wells should be verified after sampling. Record TD and water volume information in the *Low-Flow Purging and Sampling Log*.

5.2 Purging and Sampling

- 1. If using non-dedicated equipment, place the pump and support equipment at the well head and slowly lower the pump and tubing down into the monitoring well until the location of the pump intake is set at a predetermined location within the screen interval. Where possible, pre-measured tubing should be used to place the pump intake at the same depth as previous sampling events, or at a depth where there is known contamination within the screen interval. If there is no previous information for the well, the pump intake should be placed at the middle (or slightly above the middle) of the screen interval. Record the pump depth in the *Low-Flow Purging and Sampling Log*.
- 2. Measure depth to water to the nearest 0.01 feet relative to the reference measuring point on the TOC with an electronic water level indicator. Record depth to groundwater information in the *Low-Flow Purging and Sampling Log*. Leave water level indicator in the well.
- 3. Connect the discharge line from the pump to a flow-through cell that at a minimum measures temperature, pH, SEC, DO, and ORP. Turbidity measurements can be made using a separate turbidity meter. The discharge line from the flow-through cell must be directed to a container to hold purge water collected during purging and sampling of the well.
- 4. Start pumping the well at a flow rate of between 0.1 and 0.5 liters per minute (L/min) and slowly increase the flow rate. (For new wells or wells with no purging history, start at the lower end of that range.) Check the water level. Maintain a steady flow rate while maintaining a drawdown of less than 0.3 feet. (Zero drawdown is optimal, but infrequently achievable). If drawdown is greater than 0.3 feet, lower the flow rate; 0.3 feet is a goal to help guide with the flow rate adjustment. This goal will be difficult to achieve in some wells due to low hydraulic conductivities and limitations to the lowest flow rate a pump can produce while maintaining steady flow. This goal may be adjusted based on site-specific conditions and personal experience. See the Special Advisory at the end of these procedures.

5. Measure the discharge rate of the pump with a graduated cylinder and a stopwatch.

Also, measure the water level and record both flow rate and water level on the *Low-Flow Purging and Sampling Log*. Continue purging, monitor and record water level and pump rate every 3 to 5 minutes. Purging rates should be kept at minimal flow to ensure

minimal drawdown in the monitoring well.

6. A minimum of one tubing volume (including the volume of the water in the pump and flow cell) must be purged prior to recording the water quality indicator parameters. After this has been accomplished, monitor and record the water quality indicator parameters every three to five minutes in the *Low-Flow Purging and Sampling Log*. Stable readings of temperature, pH, SEC, DO, turbidity and ORP indicate when a representative sample can be collected. The stabilization criterion is based on three successive readings of the water quality indicator parameters as shown in Table 1. ORP may not always be an appropriate stabilization parameter and will depend on site-specific conditions. However, readings should be recorded because of its value for double-checking oxidizing conditions. The stabilization criterion is based on three successive readings of the water quality indicator parameter and will depend to site-specific conditions. However, readings are shown in Table 1.

TABLE 1: Stabilization Criteria for Water Quality Indicator Parameters							
Parameter	Stabilization Criteria						
Temperature	± 3% of reading (minimum of ±0.2° C)						
рН	± 0.1 pH units						
Specific Electrical Conductance (SEC)	± 3% S/cm						
Dissolved Oxygen (DO)	± 0.3 milligrams per liter						
Turbidity	± 10% NTUs (when turbidity is greater than 10 NTUs)						
Oxidation-Reduction Potential (ORP)	± 10 millivolts						

7. Maintain the same pumping rate or reduce slightly for sampling as necessary in order to minimize disturbance of the water column. Sampling should be collected directly from the discharge port of the pump tubing prior to passing through the flow-through cell. Disconnect the pump's tubing from the flow-through cell so that the samples are collected from the pump's discharge tubing. For samples collected for dissolved gases or VOC analyses, the pump tubing needs to be completely full of ground water to prevent the ground water from being aerated as it flows through the tubing. Generally, the sequence of the samples is immaterial unless filtered (dissolved) samples are collected. Filtered samples must be collected last (see below). All sample containers should be filled with minimal turbulence by allowing the ground water to flow from the tubing gently down the inside of the container. When filling VOC samples using volatile organic analysis (VOA) vials, a meniscus must be

formed over the mouth of the VOA vial to eliminate the formation of air bubbles and head space prior to capping. Effervescence and colorimetric reactions should be recorded in the *Low-Flow Purging and Sampling Log.*

- 8. If a filtered (dissolved) metal sample is to be collected, then an inline filter is fitted at the end of the discharge tubing and the sample is collected after the filter. The inline filter must first be flushed in accordance with manufacturer's recommendations and if there are no recommendations for flushing, a minimum of 0.5 to 1.0 liter of groundwater from the monitoring well must pass through the filter prior to sampling. (Note: Groundwater filter cartridges are dedicated sampling equipment. A new cartridge should be used at each sampling location. Do not attempt to clean filter cartridges. If the filter becomes clogged or groundwater flow is too slowed, remove and replace with a new filter cartridge.)
- 9. For non-dedicated systems, remove the pump from the monitoring well. Decontaminate the pump and dispose of the tubing. For dedicated systems, disconnect the tubing that extends from the plate at the wellhead (or cap) and discard after use.
- 10. Close and lock the well.

<u>Special Advisory:</u> If a stabilized drawdown in the well can't be maintained at 0.3 feet and the water level is approaching the top of the screened interval, reduce the flow rate or turn the pump off (for 15 minutes) and allow for recovery. It should be noted whether or not the pump has a check valve. A check valve is required if the pump is to be shut off during purging. Under no circumstances should the well be pumped dry. Begin pumping at a lower flow rate, if the water draws down to the top of the screened interval again, turn pump off and allow for recovery. If two tubing volumes (including the volume of water in the pump and flow cell) have been removed during purging, then sampling can proceed next time the pump is turned on. This information should be noted in the *Low-Flow Purging and Sampling Log.* This behavior may necessitate an alternative purging and sampling procedure for subsequent sampling events.

5.3 Equipment Decontamination

The electronic water level indicator and the water quality meters will be decontaminated by the following procedures:

- 1. The water level indicator will be hand washed with phosphate-free detergent and a scrubber, then thoroughly rinsed with distilled water, or steam-cleaned.
- 2. Water quality meter sensors and flow-through cell will be rinsed with distilled water between sampling locations. No other decontamination procedures are necessary or recommended for these meters since they are sensitive instruments. After the sampling event, the flow-through cell and sensors must be cleaned and maintained per the manufacturer's requirements.

Upon completion of the groundwater sample collection the sampling pump must be decontaminated between monitoring wells. The pump and discharge line including support cable and electrical wires which were in contact with the groundwater in the well

casing must be decontaminated by the following procedure:

- 1. The outside of the pump, tubing, support cable and electrical wires must be pressuresprayed with soapy water, tap water and distilled water. Spray outside of tubing and pump until water is flowing off of tubing with each rinse. Use bristle brush to help remove visible dirt and contaminants.
- 2. Place the sampling pump in a bucket or in a short cylinder or well casing (4-inch diameter) with one end capped. The pump placed in this device must be completely submerged in the water. A small amount of phosphate-free detergent must be added with the potable (tap) water.
- 3. Remove the pump from the bucket or 4-inch casing and scrub the outside of the pump housing and cable.
- 4. Place pump and discharge line back in the container, start pump and re-circulate soapy water for approximately 2 minutes.
- 5. Re-direct discharge line to a 55-gallon drum. Continue to add 5 gallons of potable (tap) water.
- 6. Turn pump off and place pump into a second bucket of potable (tap) water. Continue to add 5 gallons of tap water.
- 7. Turn off and place pump into a third bucket which contains distilled/deionized water, continue to add 3 to 5 gallons of water.
- 8. If hydrophobic contaminants are present (such as separate phase (i.e. LNAPL or DNAPL, high levels of PCBs, etc.) an additional decontamination step, or steps, may be required.
- 9. Decontamination water will be collected and stored on-site for future disposal by the client unless other arrangements have been made.

6 Quality Control Samples

All field Quality Control (QC) samples must be prepared the same as primary samples with regard to sample volume, containers, and preservation. The sample handling and chain-of-custody procedures for the QC samples will be identical to the primary samples. The following are QC samples that may be collected during groundwater sampling:

- A field duplicate is an independent sample collected as close as possible to the same time that the primary sample is collected and from the same source. Field duplicates are used to document sample precision. Field duplicates will be labeled and packaged in the same manner as primary samples so that the laboratory cannot distinguish between the primary sample and the duplicate sample. Field duplicates are analyzed for the same suite of parameters as the primary samples. The frequency of analysis of field duplicates is generally one for every 20 primary samples, but may vary depending on project requirements.
- Equipment blanks are obtained by running distilled or deionized water over or through the sample collection equipment after it has been decontaminated, and capturing the water in

the appropriate sample containers for analysis. Equipment blanks are analyzed for the same suite of parameters as the primary samples. The frequency of analysis of equipment blanks is generally one for every day that non-dedicated sampling equipment is used, but may vary depending on project requirements.

- Field blanks are used to assess the presence of contaminants arising from field sampling procedures. Field blank samples are obtained by filling a clean sampling container with reagent-grade deionized water. Field blanks are analyzed for the same suite of parameters as the primary samples. Field blanks may or may not be incorporated into a groundwater sampling plan depending on project requirements.
- Trip blanks are sample containers that are used to evaluate sample cross-contamination of VOCs during shipment. For groundwater sampling, trip blanks consist of hydrochloric acidpreserved, analyte-free, deionized water prepared by the laboratory in VOA vials that will be carried to the field, stored with the samples, and returned to the laboratory for VOC analysis. Generally, one trip blank is required to accompany each sample shipping container or cooler that contains samples for VOC analysis; however, this may vary depending on project requirements.

7 Sample Handling and Custody

Samples will be collected, handled, and stored in such a manner that they are representative of their original condition and chemical composition. Identification of samples and maintenance of custody are important elements that must also be utilized to ensure samples characterize site conditions. All samples will be properly identified and maintained under chain-of-custody protocol to protect sample integrity. The following sections discuss the sample handling and custody requirements.

7.1 Sample Identification

To maintain consistency, a sample identification convention including unique identifiers for all groundwater and QC samples must be developed and followed throughout the project. The sample identifiers will be entered onto the sample labels, field forms, chain-of-custody forms, and other records documenting sampling activities.

7.2 Sample Labels

A sample label will be affixed to all sample containers sent to the analytical laboratory. Field personnel will complete an identification label for each sample with the following information written in waterproof, permanent ink:

- Client and project number;
- Sample location and depth, if relevant;
- Unique sample identifier;
- Date and time sample collected;
- Filtering performed, if any;
- Preservative used, if any;
- Name or initials of sampler; and

• Analyses or analysis code requested.

The use of pre-printed sample labels is preferred in order to reduce sample misidentification problems due to transcription errors. Sample labels must be completed and affixed to the sample container in the field at the time of sample collection.

If errors are made on a sample label, corrections will be made by drawing a single line through the error and recording the correct information. Corrections will be dated and initialed.

7.3 Containers, Preservation, and Hold Time

Each lot of preservative and sampling containers will be certified as contaminant-free by the supplier. All preserved samples will be clearly identified on the sample label and *Chain-of-Custody* form. If samples requiring preservation are not preserved, field records will clearly specify the reason for the discrepancy.

Chemical activity continues in the sample until it is either analyzed or preserved. Once the sample has been preserved, the sample may be held for a period of time before analysis. The time from the collection of the sample to the analysis is defined as the holding time. The holding time varies depending on the media being sampled and the analyses being performed. The collection, preservation, and analysis of samples must be conducted to avoid exceeding relevant holding times.

7.4 Sample Handling and Transport

Proper sample handling techniques are used to ensure the integrity and security of the samples. Samples for field measured parameters will be analyzed immediately in the field and recorded in the appropriate field forms. Samples for laboratory analysis will be transferred immediately to appropriate laboratory supplied containers in accordance with the following sample handling protocols:

- Don clean gloves before touching any sample containers, and take care to avoid direct contact with the sample;
- Samples will be quickly observed for color, appearance, and composition and recorded as necessary;
- The sample container will be labeled before or immediately after sampling;
- Sample containers and liners will be capped with Teflon[™]-lined caps before being placed in Ziploc[™]-type plastic bags. The samples will be placed in an ice chest kept at 4 °C for transport to the laboratory;
- All sample lids will stay with the original containers, and will not be mixed;
- Sample bottles will be wrapped in bubble wrap as necessary to minimize the potential for breakage during shipment; and
- The *Chain-of-Custody* form will be placed in a separate plastic bag and taped to the cooler lid or placed inside the cooler. A custody seal will be affixed to the cooler if the samples are to be shipped by commercial carrier. For shipped samples, U.S. Department of Transportation shipping requirements will be followed and the sample shipping receipt will

be retained in the project files as part of the permanent Chain-of-Custody document.

7.5 Sample Chain-of-Custody

Sample chain-of-custody procedures will be used to maintain and document sample integrity during collection, transportation, storage, and analysis. A sample is considered to be under the control of, and in the custody of, the responsible person if the samples are in their physical possession, locked or sealed in a tamper-proof container, or stored in a secure area.

The *Chain-of-Custody* form provides an accurate written record that traces the possession of individual samples from the time of collection in the field until they are accepted at the analytical laboratory. The *Chain-of-Custody* form also documents the samples collected and the analyses requested. The sampler will record the following information on the *Chain-of-Custody* forms:

- Client and project number;
- Name or initials and signature of sampler;
- Name of destination analytical laboratory;
- Name and phone number of Project Leader in case of questions;
- Unique sample identifier for each sample;
- Data and time of collection for each sample;
- Number and type of containers included for each sample;
- Analysis or analyses requested for each sample;
- Preservatives used, if any, for each sample;
- Sample matrix for each sample;
- Any filtering performed, if applicable, for each sample;
- Signatures of all persons having custody of the samples;
- Dates and times of transfers of custody;
- Shipping company identification number, if applicable; and
- Any other pertinent notes, comments, or remarks.

Blank spaces on the *Chain-of-Custody* will be crossed out and initialed by the field sampler between the last sample listed and the signatures at the bottom of the sheet.

The field sampler will sign the *Chain-of-Custody* and will record the time and date at the time of transfer to the laboratory or an intermediate person. A set of signatures is required for each relinquished/received transfer, including internal transfer. The original imprint of the *Chain-of-*

Custody will accompany the sample containers and a duplicate copy will be kept in the project file.

If the samples are to be shipped to the laboratory, the original *Chain-of-Custody* relinquishing the samples will be sealed inside a plastic bag within the ice chest, and the chest will be sealed with

custody tape that has been signed and dated by the last person listed on the *Chain-of- Custody*. U.S. Department of Transportation shipping requirements will be followed and the sample shipping receipt will be retained in the project files as part of the permanent *Chain-of- Custody* document. The shipping company (e.g., Federal Express, UPS) will not sign the *Chain- of-Custody* forms as a receiver; instead the laboratory will sign as a receiver when the samples are received.

8 Field Documentation

Information collected during groundwater sampling may be recorded on individual field forms. If the project requires it, a project-specific Field Logbook may replace any of the individual field forms with the exception of the *Chain-of-Custody* form. Following review by the Project Manager, the original field records will be kept in the project file. The following forms may be used to document the field activities:

- Field Investigation Daily Log
- Water Level Measurement Log
- Low-Flow Purging and Sampling Log
- Equipment Calibration Log
- Chain-of-Custody

The *Field Investigation Daily Log* will be completed for each day of fieldwork containing (at a minimum) the times and descriptions of the work performed, the activities of the drillers and any other subcontractors or visitors on-site, arrival and departure times for all involved, and any other pertinent information. For larger projects, or when otherwise deemed appropriate by the Project Manager, this information may alternatively be recorded in a Field Logbook. In these cases, a separate Field Logbook must be used for each project or site.

The *Water Level Measurement Log* will be used to record water level measurements for all wells prior to commencement of groundwater sampling. The type, serial number, and calibration date for the water level measuring device will be included on this form. Additionally, this form will be used to record general observations of the conditions of the wells, wellheads, well boxes, and/or monuments.

The *Low-Flow Purging and Sampling Log* will be used to record the details of purging and sampling information for each well including the depth of the pump, purge rates, and volume purged from each well. This form will also be used to record all of the measurements of drawdown and water quality indicator parameters used for evaluating stabilization.

The *Equipment Calibration Log* will be used to document the calibration and status of any measuring instruments used in the field, e.g., PID/FID, water level measuring device, water quality meters, etc. The frequency and method of calibration will depend on the instrument. Any instruments used will be used in accordance with the factory-provided operating and/or service manuals.

Locations and unique identification of water samples collected from the monitoring wells will be

recorded on the *Field Investigation Daily Log*, *Low-Flow Purging and Sampling Log*, a site map, and/or other appropriate forms.

Samples names, date/times, analyses to be performed, and other pertinent information will be recorded on the *Chain-of-Custody* form (discussed in Section 7.5) as a means of identifying and tracking the samples.

9 References

United States Environmental Protection Agency (USEPA). 2002. Standard Operating Procedure for Low-Stress (Low Flow)/Minimal Drawdown Ground-Water Sample Collection.

Puls, Robert W. and Michael J. Barcelona. 1996. Low-Flow (Minimal Drawdown) Ground-Water Sampling Procedures. April.

California Environmental Agency's (Cal-EPA) Representative Sampling of Groundwater for Hazardous Substances. 2005. Guidance Manual for Ground Water Investigations, June.

Standard Operating Procedure C-3:

Monitoring Well Installation and Development

Monitoring Well Installation and Development

April 22, 2014

ENVIRON International Corporation

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1. Purpose

This standard operating procedure (SOP) is applicable to the installation and development of wells for groundwater monitoring or remediation purposes. This SOP is generic in nature and may be modified in whole or part depending on constraints presented by site conditions and equipment limitations. Modifications of methodologies will be documented in the appropriate field logbook and discussed in reports summarizing field activities. The procedures herein were prepared in accordance with applicable sections of Chapter 534 of the Nevada Administrative Code.

2. Well Installation

Prior to invasive activities, a subsurface utility check will be conducted. Wells will generally be constructed using 5- to 20-foot-long screen and sufficient riser to complete the well to, or slightly above, ground surface. The length of the well screen will be selected based on the planned use of each well and the observed lithology. Wells will be constructed using schedule 40 polyvinyl chloride (PVC) casing and 0.010 slot schedule 40 PVC well screen with a threaded bottom cap. Wells will generally be completed with a protective steel cover extending a minimum of 18 inches above the finished grade and a minimum of 5 feet below the seal. The protective cover will be equipped with a lock to protect the well against damage and unauthorized entry.

3. Filter Material

Filter material will be well-graded, clean sand (generally less than 2-percent by weight passing a No. 200 sieve and less than 5 percent by weight of calcareous material).

4. Setting Wells

Upon completion of borehole drilling, the boring will be sounded to determine the total depth, and the PVC well materials will be assembled and lowered into the boring. PVC well materials will be measured to the nearest 0.1 foot and will be assembled such that the screened interval is positioned opposite the target formation. No PVC cement or other solvents will be used. Once the well has been positioned at the desired depth, filter sand will be slowly added to the borehole to fill the annular space to a depth approximately 1 to 2 feet above the top of the well screen. During sand placement, the driller will continually measure the depth to the sand using a weighted tape measure or other device to verify that the sand does not bridge between the auger and the well screen. A minimum of two feet of bentonite chips will be added on top of the filter sand and subsequently hydrated using clean, municipal water to form a transition seal. After the bentonite has hydrated for at least 30 minutes, the depth to the top of the bentonite will be measured and recorded. A neat cement/bentonite grout will be added from the top of the bentonite; a tremie pipe will be utilized to ensure that the grout is added from the bottom, upwards. The grout will be permitted to cure for 48 hours prior to well development.

5. Well Completion

All monitoring wells and monitoring points will be completed with a protective steel cover equipped with a lock to protect the well against damage and unauthorized entry. Wells will typically be completed above grade unless they are located within parking/driving areas, or are piped to a remediation system. Wells completed aboveground will be capped with a push-on well cap and completed with a steel stick-up casing extending at least one foot above the surface pad. Wells completed below ground surface will be capped with an expandable locking well cap and completed with a flush mounted traffic rated steel cover set into a 2 foot by 2 foot concrete pad, expending one-half inch above the surface concrete or ashpalt. All wells will be labeled with a permanent marker that includes the well ID.

6. Development and Surveying

New wells will be developed after the grout has cured for a minimum of 48 hours. Wells will be developed by surging, bailing, and pumping to reduce or remove drilling-induced formation smear from the borehole walls, to remove sediment that may have accumulated during well installation, consolidate the filter pack, and to enhance the hydraulic connection between the formation target zone and the well. In most cases, a bailer or pump will be used to remove sediment and turbid water from the bottom of the well. A surge block will then be lowered up and down within the screened interval to flush the filter pack of fine sediment and remove smear from borehole walls. Following surging, the well will be bailed or pumped again to remove sediment and turbid water. Water will be removed from the well at a rate greater than the anticipated future pumping rate and water quality parameters including pH, turbidity, specific conductance and temperature will be recorded. Drawdown will also be recorded with an interface probe or water level meter. The development will proceed until sediment is removed sufficiently to achieve a turbidity measurement of 5 NTU (or less). The well installation report will specify if the target turbidity cannot be achieved.

Following well installation and completion, each well will be surveyed by a licensed surveyor to determine the location of the well and to establish the elevation at the top of casing and ground surface with reference to the site datum. Survey data will be incorporated into the database and onto the site base map.

7. Decontamination of Drilling Equipment

All drilling and well development equipment will be cleaned prior to use, and between wells. Drilling equipment will be steam cleaned, rinsed with potable water, and air dried. If equipment is not immediately put back to use, equipment will be covered with clean plastic to protect the materials from contact with dust or other contaminants. Pumps or other non-dedicated field equipment that comes into contact with impacted media will be cleaned using a non-phosphate detergent followed by a tap water rinse and a final, deionized water rinse. Decontamination water will be collected for appropriate, subsequent off-site disposal. Spent PPE or other disposable materials (e.g., tubing) will be placed into a drum for subsequent disposal.

8. Documentation

Well installation and construction activities will be recorded in the field notebook. A well construction diagram will be completed for each well, reviewed by appropriate personnel for completeness and accuracy, and filed electronically in the project file. The CQA Officer will complete and submit an Well Completion form for each well.

9. References

ENVIRON. 2014. Quality Assurance Project Plan, Nevada Environmental Response Trust, Henderson, Nevada. January 24.

Nevada Division of Environmental Protection. 2012. Nevada Administrative Code, Chapter 534 – Underground Water and Wells. June.

Standard Operating Procedure C-4:

Photoionization Detector (PID) Screening

Photoionization Detector (PID) Screening

April 22, 2014

ENVIRON International Corporation

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1. Purpose

This standard operating procedure (SOP) is applicable to the use of a photoionization detector/flame ionization detector (PID/FID) instrument during soil sampling activities. The methodology is generic in nature and may be modified in whole or part to meet the handling and analytical requirements of the contaminants of concern, as well as the constraints presented by site conditions and equipment limitations. Modifications of sampling methodologies will be documented in the appropriate field logbook and discussed in reports summarizing field activities and analytical results. For the purposes of this procedure, soils are those mineral and organic materials not submerged in water for an extended period of time sufficient to support aquatic life.

2. Equipment/Apparatus

Equipment needed for PID/FID screening of soil samples may include:

- PID/FID instrument
- Clear glass jar
- Aluminum foil
- Ziploc bags

3. Procedure

When using PID/FID instrument the following procedure must be used:

- Half-fill either a glass jar, or a Ziploc® baggie.
 - When using glass jars:
 - Fill jars with a total capacity of 8 oz. or 16 oz.
 - Seal each jar with one (1) or two (2) sheets of aluminum foil with the screw cap applied to secure the aluminum foil.
 - When using Ziploc® baggies:

Half fill bags from the split spoon or the excavation.

Zip to close.

- Vigorously shake the sample jars or bags for at least thirty (30) seconds once or twice in a 10- to 15-minute period to allow for headspace development.
- If ambient temperatures are below 32 degrees Fahrenheit (0 degrees Celsius) headspace development is to be within a heated vehicle or building.
- Quickly insert the PID/FID sampling probe through the aluminum foil. If plastic bags are used, unzip the corner of the bag approximately one to two inches and insert the probe or insert the probe through the plastic. Record the maximum meter response (should be within the first 2 to 5 seconds). Erratic responses should be discounted as a result of high organic vapor concentrations or conditions of elevated headspace moisture.
- Record headspace screening data from both jars or bags for comparison.

- Calibration will be checked/adjusted daily. In addition, all manufacturers' requirements for instrument calibration will be followed.
- If sample jars are re-used in the field, jars will be cleaned according to field decontamination procedures. In addition, headspace readings must be taken to ensure no residual organic vapors exist in the cleaned sample jars.
- Plastic bags will not be reused.

4. References

ENVIRON. 2014. Quality Assurance Project Plan, Nevada Environmental Response Trust, Henderson, Nevada. January 24.